Appendices



Appendix A:

Topographic Information



Appendix B: FEMA



ZONE B ZONE A5 ZONE B -----513---

*EXPLANATION OF ZONE DESIGNATIONS

EXPLANATION

 $RM7_{\times}$

•M1.5

Areas of 100-year flood; base flood elevations and flood hazard factors not determined.

Areas of minimal flooding. (No shading)
Areas of undetermined, but possible, flood hazards.
Areas of 100-year coastal flood with velocity (wave action); base flood elevations and flood hazard factors not determined.

NOTES TO USER

This map is for flood insurance purposes only; it does not nece sarily show all areas subject to flooding in the community of all planimetric features outside special flood hazard areas. For adjoining map panels, see separately printed Index To Map

INITIAL IDENTIFICATION:

SEPTEMBER 24, 1976 NOVEMBER 14, 1980

FLOOD INSURANCE RATE MAP EFF MAY 1, 1984

FLOOD INSURANCE RATE MAP REVISIONS

Refer to the FLOOD INSURANCE RATE MAP EFFECTIVE dat shown on this map to determine when actuarial rates apply t structures in the zones where elevations or depths have been estal

APPROXIMATE SCALE

400 0

NATIONAL FLOOD INSURANCE PROGRAM

FIRM FLOOD INSURANCE RATE MAP

TOWNSHIP OF WEST WINDSOR, NEW JERSEY MERCER COUNTY

PANEL 4 OF 10

COMMUNITY-PANEL NUMBER 340256 0004 C EFFECTIVE DATE:

MAY 1, 1984



TOWNSHIP OF WEST WINDSOR, NEW JERSEY MERCER COUNTY



NOVEMBER 1, 1983



Federal Emergency Management Agency

COMMUNITY NUMBER - 340256

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PUBLISHED SEPARATELY:

Flood Insurance Rate Map Index

Flood Insurance Rate Map

FLOOD INSURANCE STUDY TOWNSHIP OF WEST WINDSOR, NEW JERSEY

1.0 INTRODUCTION

1.1 Purpose of Study

This Flood Insurance Study investigates the existence and severity of flood hazards in the Township of West Windsor, Mercer County, New Jersey, and aids in the administration of the National Flood Insurance Act of 1968 and the Flood Disaster Protection Act of 1973. This study will be used to convert West Windsor to the regular program of flood insurance by the Federal Emergency Management Agency (FEMA). Local and regional planners will use this study in their efforts to promote sound flood plain management.

In some states or communities, flood plain management criteria or regulations may exist that are more restrictive or comprehensive than those on which these federally-supported studies are based. These criteria take precedence over the minimum federal criteria for purposes of regulating development in the flood plain, as set forth in the Code of Federal Regulations at 44 CFR, 60.3. In such cases, however, it shall be understood that the state (or other jurisdictional agency) shall be able to explain these requirements and criteria.

1.2 Authority and Acknowledgements

The source of authority for this Flood Insurance Study is the National Flood Insurance Act of 1968 and the Flood Disaster Protection Act of 1973.

The hydrologic and hydraulic analyses for this study were prepared by the New Jersey Department of Environmental Protection for the Federal Emergency Management Agency, under Contract No. H-3959. This work was completed in May 1980. The hydrologic and hydraulic analyses for this study were conducted by Justin & Courtney, Inc. under subcontract to the New Jersey Department of Environmental Protection.

1.3 Coordination

On March 16, 1976, an initial Consultation and Coordination Officer's (CCO) meeting, attended by representatives of the Township of West Windsor, the FEMA and the New Jersey Department of Environmental Protection (NJDEP - the study contractor), was held to identify the streams to be studied by detailed methods. A search for data was made at all levels of government.

On May 16, 1983, a final CCO meeting, attended by representatives of the Township of West Windsor, the FEMA and the study contractor, was held to review the results of the study.

2.0 AREA STUDIED

2.1 Scope of Study

This Flood Insurance Study covers the incorporated area of the Township of West Windsor, Mercer County, New Jersey. The area of study is shown on the Vicinity Map (Figure 1).

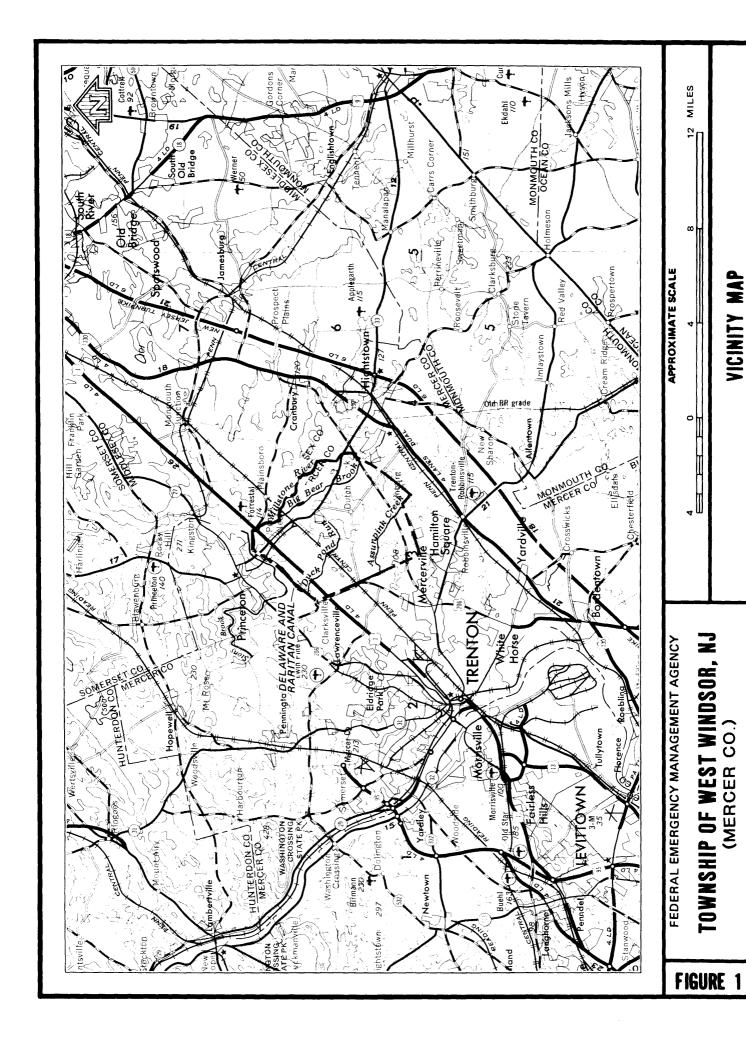
The following streams were studied by detailed methods for their entire lengths within the community: The Millstone River, Big Bear Brook, Bear Creek and Assunpink Creek. In addition, Canoe Brook, from its confluence with Big Bear Brook to just upstream of Penn-Lyle Road; Little Bear Brook, from its confluence with the Millstone River to Meadow Road; Duck Pond Run, from the corporate limits to just upstream of Penn-Lyle Road; and Bridgegroom Run, from the confluence with Assunpink Creek to Old Trenton Road were studied by detailed methods. Stony Brook, including the Delaware and Raritan Canal, was studied in detail for its entire length affecting the community. The areas studied by detailed methods were selected with priority given to all known flood hazard areas and areas of projected development and proposed construction for the next five years, through May 1985.

The remaining portion of Bridegroom Run was studied by approximate methods. In addition, an unnamed tributary to Beaver Brook and an unnamed tributary to the Millstone River were studied by approximate methods. Approximate methods of analysis were used to study those areas having low development potential and minimal flood hazards as identified at the initiation of the study. The scope and methods of study were proposed to and agreed upon by the FEMA.

2.2 Community Description

The Township of West Windsor is located in Mercer County in west-central New Jersey. The Township of West Windsor is bordered by the Town of Plainsboro and the Township of Princeton to the north, the Township of East Windsor to the east, the Townships of Washington and Hamilton to the south and the Township of Lawrence to the west. The 1960 population of the community was 4,016 and the 1970 population was 6,431 (Reference 1). The 1975 population was estimated to be approximately 8,100.

Development within the township is primarily agricultural and residential, with small areas of commercial development and light industry. Recently, there has been a considerable increase in



residential development in West Windsor. The township was formed on February 9, 1797 from the Township of Windsor in Middlesex County. Since then, there have been several changes made to the corporate limits of the community.

The primary routes of transportation in the township are Washington Road-Princeton Junction-Hightstown Road, running northwest and southeast through Princeton Junction; Charlesville-Grovers Mill Road, running northeast and southwest through Princeton Junction; and U. S. Route 1, running northeast to Brunswick and southwest to Trenton. Washington Road connects Princeton Junction with U. S. Route 1 and the Borough of Princeton. The main line of Conrail from Philadelphia to New York crosses the west-central portion of the township.

Most of the residential areas in the community are located outside the flood plains of the Millstone River, Big Bear Brook, Bear Brook, Stony Brook and Assunpink Creek. Residential developments are located in the flood plains of Canoe Brook and Duck Pond Run at elevations of 7 to 10 feet above the normal stream level. One residential development is partially located within the flood plain of Bridesgroom Run at elevations of 3 to 4 feet above the normal stream level.

The Millstone River flows east to west through West Windsor and forms the northeast corporate limits of the township. The Millstone River has a drainage area of 99.0 square miles at the confluence of Stony Brook.

Big Bear Brook flows southeast to northwest through the northern portion of the township. Big Bear Brook has a drainage area of 12.0 square miles at its confluence with the Millstone River.

Bear Creek flows south to north through the eastern portion of the Township of West Windsor and has a drainage area of 3.2 square miles at its confluence with Big Bear Brook.

Canoe Brook flows south to north through the north-central portion of the township and has a drainage area at its confluence with Big Bear Brook of 0.8 square mile.

Little Bear Brook flows from southwest to northeast through the western portion of the township. Little Bear Brook has a drainage area of 3.3 square miles at its confluence with the Millstone River.

Duck Pond Run flows east to west through the western portion of the township and has a drainage area of 5.8 square miles at its confluence with Stony Brook in the western portion of the community.

Stony Brook flows west to east along the northwest corporate limits of the township. Stony Brook has a drainage area of 58.7 square miles at its confluence with the Millstone River.

Assumpink Creek flows east to west in the southern end of the township. The drainage area of Assumpink Creek is 33.2 square miles at the downstream corporate limits.

Bridegroom Run, with a drainage area of 1.2 square miles, flows east to west in the southern part of the township.

The climate of the area is continental due to the prevailing westerly winds. The average annual temperature is approximately 53 degrees Fahrenheit (°F), with temperature extremes ranging from approximately 5°F to 95°F. The average annual precipitation is approximately 33 inches. Most storm systems affecting the study area originate in the west with occasional coastal storms affecting the area from the east.

2.3 Principal Flood Problems

Low-lying areas along the streams studied by detailed methods are subject to flooding within the Township of West Windsor. The flood of record for the area was recorded along the Millstone River on July 21, 1975, at the Plainsboro gaging station (No. 014010000 with 23 years of record). This flood had a discharge of 3,970 cubic feet per second. The same storm which caused this flow also caused flooding of Little Bear Brook, Canoe Brook, Big Bear Brook, Duck Pond Run, Bridegroom Run and Assunpink Creek. Floods have been recorded along Stony Brook on September 21, 1938, August 13, 1955, January 9, 1964 and July 14, 1975. The most severe flooding usually occurs after intense rainfall or after repeated heavy rainfall. Potential 100- and 500-year flood heights in the township are shown in Figures 2 and 3.

2.4 Flood Protection Measures

There are no completed flood protection measures on the streams in the Township of West Windsor. The Assumpink Creek Flood Prevention Project under direction of the Soil Conservation Service has completed four flood protection dams upstream of the township in Monmouth County. An additional flood control dam (Site-20) is under construction in the eastern part of the township. These dams will help to reduce flood flow on Assumpink Creek further downstream within the township.

In addition, non-structural measures of flood protection aid in the reduction of flood hazards within the township. New development is subject to the requirements of a township ordinance restricting



Figure 2 - Potential 100- and 500-year flood heights along Duck Pond Run on the upstream side of the U. S. Route 1 bridge.



Figure 3 - Potential 100- and 500-year flood heights along Big Bear Brook at Cranbury Road, looking upstream.

construction in the flood plain areas in accordance with FEMA regulations. In an effort to minimize flood damages, the Division of Water Resources of the NJDEP has adopted rules, regulations and minimum standards concerning development and use of land within the floodway.

3.0 ENGINEERING METHODS

For the flooding sources studied in detail in the community, standard hydrologic and hydraulic study methods were used to determine the flood hazard data for this study. Flood events of a magnitude which are expected to be equalled or exceeded once on the average during any 10-, 50-, 100-, or 500-year period (recurrence interval) have been selected as having special significance for flood plain management and for flood insurance premium ates. These events, commonly termed the 10-, 50-, 100-, and 500-year floods, have a 10-, 2-, 1-, and 0.2-percent chance, respectively, of being equalled or exceeded during any year. Although the recurrence interval represents the long-term average period between floods of a specific magnitude, rare floods could occur at short intervals or even within the same year. experiencing a rare flood increases when periods greater than one year are considered. For example, the risk of having a flood which equals or exceeds the 100-year flood (one-percent chance of annual occurrence) in any 50-year period is about 40 percent (four in ten) and, for any 90-year period, the risk increases to about 60 percent (six in ten). The analyses reported here reflect flooding potentials based on conditions existing in the community at the time of completion of this study. Maps and flood elevations will be amended periodically to reflect future changes.

3.1 Hydrologic Analyses

Hydrologic analyses were carried out to establish the peak dischargefrequency relationships for floods of the selected recurrence intervals for each flooding source studied in detail affecting the community.

The discharges for the 10-, 50-, 100- and 500-year recurrence interval floods for the Millstone River were based on stream gage records. The peak discharges were calculated using a log Pearson Type III analysis of annual peak flow data and the natural gage skew (Reference 2). Flows calculated for the gage (No. 01460500 with 29 years of record) located at Kingston were transposed to specific locations on the Millstone River according to the drainage area discharge formula:

$$Q_{\mathbf{T}}/Q_{\mathbf{g}} = [\mathbf{A}_{1}/\mathbf{A}_{2}]^{\mathbf{T}}$$

where Q_T is the discharge at a specific location, A_1 is the drainage area at that point, Q_g is the discharge at the gage and A_2 is the drainage area at the gage with T being the transfer exponent. A value for T of 0.75 was used for the Millstone River.

For Big Bear Brook, Bear Creek, Little Bear Brook, Duck Pond Run, Stony Brook and the upstream portions of Bridegroom Run, the hydrolgic analyses were based upon the method for estimating flood peaks shown in Special Report 38 (Reference 3). Special Report 38 was developed under a cooperative program between the NJDEP and the U. S. Geological Survey (USGS). This method is based on a multiple regression analysis used to develop mathematical relationships between flood discharges of various recurrence intervals (2-, 10-, 50- and 100-years) obtained from gaging station data and hydrologic characteristics. Flood information from 103 sites was used in this analysis. Hydrologic parameters include drainage area, main channel slope, surface storage area and an index of man-made impervious cover based upon basin population and development conditions. The 500-year discharge is extrapolated from the other frequency floods.

The Rational Method was utilized for Canoe Brook and the farthest upstream portions of Big Bear Brook and Bridegroom Run since they have drainage areas less than one square mile in area. The Rational Method obtains flood flows using the watershed area, a coefficient (C) of runoff basin on surface conditions within the watershed and the intensity of rainfall based on concentration time. Varying values of runoff coefficients (C) were used for different flood frequencies as suggested in Modified Rational Method of Estimating Flood Flows (Reference 4).

For Assumpink Creek, stream flow data was coordinated with the Flood Insurance Studies for the Townships of Washington and Lawrence and a SCS publication for the Assumpink Creek watershed (References 5, 6 and 7). For these studies, the SCS standard hydrologic model TR-20, which develops runoff hydrographs and performs reach and reservoir routings, was used to determine flood discharges (Reference 8). Calibration for the model was provided by data from 53 years of record at USGS gage No. 01464000 (Reference 9). This gage was installed in 1923 on Assumpink Creek at Trenton, New Jersey.

A summary of the drainage area-peak discharge relationhips for the streams studied by detailed methods is shown in Table 1, "Summary of pischarges."

TABLE 1 - SUMMARY OF DISCHARGES

	DRAINAGE AREA	P	EAK DISCH	ARGES (cis)
FLOODING SOURCE AND LOCATION	(sq. miles)	10-YEAR	50-YEAR	100-YEAR	500-YEAR
MILLSTONE RIVER At confluence with Stony					
Brook	99.0	4,485	7,570	9,030	12,950

TABLE 1 - SUMMARY OF DISCHARGES - continued

	DRAINAGE AREA	P	EAK DISCH	ARGES (cfs)
FLOODING SOURCE AND LOCATION	(sq. miles)	10-YEAR	50-YEAR	100-YEAR	500-YEAR
MILLIAMOND DIVID					
MILLSTONE RIVER					
(continued)					
Above confluence of Little	04.0	4 000		7 000	44 045
Bear Brook	81.8	4,230	6,555	7,820	11,215
Above confluence of Big	45.0				
Bear Brook	65.8	3,600	5,575	6,650	9,535
Above conflunce of Cranbury					
Brook	42.7	2,600	4,025	4,800	6,885
At upstream corporate					
limits	39.3	2,445	3,785	4,515	6,475
BIG BEAR BROOK					
At confluence with Mill-					
stone River	12.0	875	1,430	1,745	2,710
Above confluence of Canoe	,		.,	.,	_,
Brook	11.7	765	1,250	1,530	2,360
Above confluence of Bear		. • •	.,	.,000	_,,500
Creek	3.3	280	480	600	900
CI eek	3.3	200	400	000	900
BEAR CREEK					
At confluence with Big					
Bear Brook	3.2	280	480	600	900
CANOE BROOK					
At confluence with Big					
Bear Brook	0.8	285	445	520	720
Bear Brook	0.0	203	333	320	720
LITTLE BEAR BROOK					
At confluence with Mill-					
stone River	3.3	120	205	250	385
DUAY DOND DVD					
DUCK POND RUN					
At confluence with Stony	. .	255	500	500	700
Brook	5.8	355	520	590	780
At U. S. Route 1	5.2	335	485	560	740
STONY BROOK					
At confluence with Mill-					
stone River	58.7	6,220	8,930	10,200	13,900
At confluence with Duck		•	-	÷	-
Pond Run	55.7	6,050	8,690	10,010	13,530
		•	•	•	•

TABLE 1 - SUMMARY OF DISCHARGES - continued

	DRAINAGE AREA	P	EAK DISCH	ARGES (cfs)
FLOODING SOURCE AND LOCATION	(sq. miles)	10-YEAR	50-YEAR	100-YEAR	500-YEAR
ASSUNPINK CREEK At downstream corporate limits	33.2	1,510	2,170	2,480	3,250
BRIDEGROOM RUN At confluence with					
Assunpink Creek	1.2	315	530	660	1,045

3.2 Hydraulic Analyses

Analyses of the hydraulic characteristics of the flooding sources studied in detail were carried out to provide estimates of the elevations of floods of the selected recurrence intervals along each of these flooding sources.

Overbank cross-section data for the streams studied by detailed methods were obtained from aerial photographs (References 10 and 11). The below-water sections were obtained from field survey. All bridges and culverts were surveyed to obtain elevatic data and structural geometry. Cross sections were located at close intervals above and below bridges and culverts in order to compute the significant backwater effects of these structures.

Channel roughness factors (Manning's "n") for the streams studied by detailed methods were assigned on the basis field inspection of the streams and flood plain areas. The channel and overbank "n" values for the streams studied by detailed methods are shown in the following tabulation:

Stream	Channel "n"	Overbank "n"
Millstone River	0.030-0.040	0.050-0.110
Big Bear Brook	0.040-0.050	0.060-0.120
Bear Creek	0.040	0.110
Canoe Brook	0.030-0.040	0.060-0.110
Little Bear Brook	0.035-0.037	0.090-0.110
Duck Pond Run	0.012-0.050	0.060-0.120
Stony Brook	0.032-0.060	0.080-0.120
Assunpink Creek	0.030-0.035	0.090-0.100
Bridegroom Run	0.013-0.035	0.090-0.100

Water-surface elevations of the floods of the selected recurrence intervals were computed through the use of the U. S. Army Corps of Engineers HEC-2 step-backwater computer program for all the streams studied by detailed methods with the exception of Assunpink Creek and the lower portion of Bridegroom Run (Reference 12).

For Assumpink Creek and the lower portion of Bridegrooom Run, elevation-discharge relationships were established using the SCS water-surface profile computer program, WSP-2 (Reference 7).

Starting water-surface elevations for the Millstone River were taken from the Flood Insurance Study for the Town of Plainsboro (Reference 13). Starting water-surface elevations for Big Bear Brook and Little Bear Brook were taken at their confluences with the Millstone River. For Bear Creek and Canoe Brook, the starting water-surface elevations were taken at their confluences with Big Bear Brook. The starting water-surface elevations for Duck Pond Run were taken at its confluence with Stony Brook. The starting water-surface elevations for Stony Brook were taken at its confluence with the Millstone River. The starting water-surface elevations for Assumpink Creek were taken from the Flood Insurance Study for the Township of Hamilton (Reference 14). The starting water-surface elevations for Bridegroom Run were taken at its confluence with Assumpink Creek.

Flood profiles were drawn showing computed water-surface elevations to an accuracy of 0.5 foot for floods of the selected recurrence intervals. Locations of selected cross sections used in the hydraulic analyses are shown on the Flood Profiles (Exhibit 1). For stream segments for which a floodway is computed (Section 4.2), selected cross-section locations are also shown on the Flood Boundary and Floodway Map (Exhibit 3).

For the streams studied by approximate methods, the 100-year flood elevations were determined using depth-discharge-frequency relations for coastal and non-coastal plain sites in New Jersey (Reference 3).

All elevations used in this study are referenced to the National Geodetic Vertical Datum of 1929 (NGVD), formerly referred to as Sea Level Datum of 1929. Locations of the elevation reference marks used in the study are shown on the maps.

The hydraulic analyses for this study are based on the effects of unobstructed flow. The flood elevations shown on the profiles are valid only if hydraulic structures remain unobstructed and do not fail.

4.0 FLOOD PLAIN MANAGEMENT APPLICATIONS

The National Flood Insurance Program encourages state and local governments to adopt sound flood plain management programs. Therefore, each Flood Insurance Study includes a flood boundary map designed to assist communities in developing sound flood plain management measures.

4.1 Flood Boundaries

In order to provide a national standard without regional discrimination, the 100-year flood has been adopted by the FEMA as the base flood for purposes of flood plain management measures. The 500-year flood is employed to indicate additional areas of flood risk in the community. For each stream studied in detail, the boundaries of the 100- and 500-year floods have been delineated using the flood elevations determined at each cross section; between cross sections, the boundaries were interpolated using photogrammetric maps a scale of 1:2,400 with contour intervals of 5 and 2 feet, respectively (References 10 and 11). In cases where the 100- and 500-year flood boundaries are close together, only the 100-year boundary has been shown.

For the streams studied by approximate methods, the 100-year flood boundary was delineated using the Flood Hazard Boundary Map of the Township of West Windsor and USGS topographic maps enlarged to a scale of 1:4,800 with a contour interval of 20 feet (References 15 and 16).

The boundaries of the 100- and 500-year floods are shown on the Flood Boundary and Floodway Map (Exhibit 3). Small areas within the flood boundaries may lie above the flood elevations and, therefore, may not be subject to flooding. Owing to limitations of the map scale and lack of detailed topographic data, such areas are not shown.

4.2 Floodways

Encroachment on flood plains, such as artificial fill, reduces the flood-carrying capacity, increases the flood heights of streams, and increases flood hazards in areas beyond the encroachment itself. One aspect of flood plain management involves balancing the economic gain from flood plain development against the resulting increase in flood hazard. For purposes of the Flood Insurance Program, the concept of a floodway is used as a tool to assist local communities in this aspect of flood plain management. Under this concept, the area of the 100-year flood is divided into a floodway and a floodway fringe. The floodway is the channel of a stream plus any adjacent flood plain areas that must be kept free of encroachment in order that the 100-year flood can be carried

without substantial increases in flood heights. Minimum standards of the FEMA limit such increases in flood heights to 1.0 foot, provided that hazardous velocities are not produced. However, the State of New Jersey has established criteria limiting the increase in flood heights to 0.2 foot. Thus, floodways having no more than a 0.2-foot surcharge have been delineated for this study. The floodways in this report are presented to local agencies as minimum standards that can be adopted or that can be used as a basis for additional studies.

The floodways presented in this study were computed on the basis of equal conveyance reduction from each side of the flood plains. The results of these computations are tabulated at selected cross sections for each stream segment for which a floodway is computed (Table 2).

The floodways for Assumpink Creek and Bridegroom Run were taken from a flood hazard analysis of the Assumpink Creek Watershed prepared by the SCS (Reference 7).

As shown on the Flood Boundary and Floodway Map (Exhibit 3), the floodway widths were determined at cross sections; between cross sections, the boundaries were interpolated. In cases where the boundaries of the floodway and the 100-year flood are either close together or collinear, only the floodway boundary has been shown. Portions of the floodway widths for the Millstone River, Stony Brook and Assumpink Creek extend beyond the corporate limits.

The area between the floodway and the boundary of the 100-year flood is termed the floodway fringe. The floodway fringe thus encompasses the portion of the flood plain that could be completely obstructed without increasing the water-surface elevation of the 100-year flood by more than 0.2 foot at any point. Typical relationships between the floodway and the floodway fringe and their significance to flood plain development are shown in Figure 4.

Near the mouths of streams studied in detail, floodway computations are made without regard to flood elevations on the receiving water body. Therefore, "With Floodway" elevations presented in Table 2 for certain downstream cross sections of Little Bear Brook and Duck Pond Run are lower than the regulatory flood elevations in that area, which must take into account the 100-year flooding due to backwater from other sources.

5.0 INSURANCE APPLICATION

In order to establish actuarial insurance rates, the FEMA has developed a process to transform the data from the engineering study into flood insurance

Z	INCREASE		0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.2	0.2	0.2	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.2	0.1	0.1	0.0	
BASE FLOOD SURFACE ELEVATION	WITH FLOODWAY NGVD)		57.9	58.7	6.09	61.0	61.3	61.7	62.1	62.5	62.6	62.7	65.0	65.0	65.2	65.2	65.4	65.6	65.7	66.1	66.4	67.0	67.3	67.8	
BASE F WATER SURFAC	WITHOUT FLOODWAY (FEET		57.8	58.6	8.09	6.09	61.2	61.6	62.0	62.3	62.4	62.5	64.9	64.9	65.1	65.1	65.3	65.5	65.6	0.99	66.2	6.99	67.2	67.8	
X	REGULATORY		57.8	58.6	8.09	6.09	61.2	61.6	62.0	62.3	62.4	62.5	64.9	64.9	65.1	65.1	65.3	65.5	65.6	0.99	66.2	6.99	67.2	67.8	
	MEAN VELOCITY (FEET PER SECOND)		1.3	1.5	1.3	2.0	1.6	1.7	1.3	8.0	1.2	8.0	9.0	0.7	6.0	1.0	1.3	1.1	1.4	2.0	2.0	1.4	1.9	2.0	
FLOODWAY	SECTION AREA (SQUARE FEET)		7,131	6,047	6,749	4,570	5,651	5,407	6,101	9,613	6,428	9,419	11,864	689'9	5,609	4,654	3,744	4,492	3,389	2,380	2,391	3,504	2,567	2,441	
	WIDTH 2 (FEET)		1090/980	960/640	750/645	600/400	770/590	830/100	870/190	1330/210	910/485	1510/1050	1890/420	930/530	870/205	729/439	568/223	707/257	08/059	480/370	460/195	670/235	780/380	765/300	
RCE	DISTANCE 1		165	1,485	2,045	3,195	4,395	5,455	6,355	8,015	6,985	10,955	11,965	12,925	13,445	14,345	15,575	17,135	18,255	19,595	20,415	20,745	21,685	21,945	
FLOODING SOURCE	CROSS SECTION	Millstone River	Ą	В	υ	Q	ш	ᄕᅭ	ტ	н	н	ט	×	ы	Σ	Z	0	Q.	O.	ж	တ	H	D	Δ	

1Feet above corporate limits 2width/width within corporate limits

FEDERAL EMERGENCY MANAGEMENT AGENCY

TOWNSHIP OF WEST WINDSOR, NJ

(Mercer Co.)

FLOODWAY DATA

MILLSTONE RIVER

f	1																							
2	INCREASE		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.1	0.1	0.1	0.1	0.1	0.0	0.0	0.0	0.0	0.1	
FLOOD CE ELEVATION	WITH FLOODWAY NGVD)		68.4	68.8	0.69	69.7	6.69	70.0	70.2	70.9	71.4	71.9		64.9	64.9	64.9	65.0	65.0	67.3	67.4	67.4	67.5	8.89	
BASE FLC	WITHOUT FLOODWAY (FEET		68.4	68.8	0.69	69.7	6.69	70.0	70.2	6.07	71.4	71.9		64.8	64.8	64.8	64.9	64.9	67.3	67.4	67.4	67.5	68.7	
W	REGULATORY		68.4	68.8	0.69	69.7	6.69	70.0	70.2	70.9	71.4	71.9		64.8	64.8	64.8	64.9	64.9	67.3	67.4	67.4	67.5	68.7	
	MEAN VELOCITY (FEET PER SECOND)		1.2	1.5	2.0	6.0	6.0	1.3	2.1	2.4	2.1	1.6		0.5	9.0	8.0	8.0	9.0	0.4	0.5	6.0	0.8	9.0	
FLOODWAY	SECTION AREA (SQUARE FEET)		3,901	3,284	2,345	5,446	5,211	3,732	2,334	2,040	2,322	3,077		3,385	3,071	2,235	2,188	3,005	4,279	3,236	1,962	2,154	2,878	
	WIDTH (FEET)		875/5553	755/4353	650/653	1060/5503	936/3263	720/3803	663/1633	800/6903	770/5753	680/953		352	341	253	273	337	532	491	411	495	160	
RCE	DISTANCE		22,7951	23,8151	24,4651	25, 7651	26,7651	27,775	28,7151	29,9151	30,1051	31, 1851		1,7052	2,9602	3,2452	3,9552	4,7752	5,0132	6,5132	7,5332	8,2132	8,6152	
FLOODING SOURCE	CROSS SECTION	Millstone River (continued)	W	×	7	2	AA	AB	AC	AD	AE	AF	Big Bear Brook	A	щ	υ	Ω	ш	Гч	U	H	н	ם	

1 Feet above corporate limits

2Feet above confluence with Millstone River

 $^3 \text{Width/width within corporate limits}$

TOWNSHIP OF WEST WINDSOR, NJ (Mercer Co.)

FEDERAL EMERGENCY MANAGEMENT AGENCY

FLOODWAY DATA

MILLSTONE RIVER AND BIG BEAR BROOK

Z	INCREASE			0.1	0.1	0.1	0.1	0.1	0.1	0.0	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1		
BASE FLOOD SURFACE ELEVATION	WITH FLOODWAY NGVD)			6.89	6.89	70.8	71.9	72.0	72.6	73.0	74.6	74.9	75.3	76.4	77.7	79.3	9.62	80.3	80.8	82.4	82.4		
BASE FLOOD WATER SURFACE EL	WITHOUT FLOODWAY (FEET			8.89	8.89	70.7	71.8	71.9	72.5	73.0	74.5	74.8	75.2	76.3	77.6	79.2	79.5	80.2	80.7	82.3	82.3		
W	REGULATORY			68.8	68.8	70.7	71.8	71.9	72.5	73.0	74.5	74.8	75.2	76.3	77.6	79.2	79.5	80.2	80.7	82.3	82.3		
	MEAN VELOCITY (FEET PER SECOND)			9.0	1.9	1.3	6.0	1.4	1.8	4.4	1.3	1.1	1.5	1.7	1.4	0.7	1.6	1.4	2.3	0.5	1.5		
FLOODWAY	SECTION AREA (SQUARE FEET)			3,091	920	1,340	1,847	1,250	948	398	1,318	1,587	1,176	1,055	1,285	2,240	1,070	1,167	729	2,118	714	 	
	WIDTH (FEET)			453	333	320	382	341	251	120	262	297	277	309	393	537	270	251	230	405	130		
RCE	DISTANCE			9,915	11,115	12,615	12,815	13,175	14,085	14,316	15,462	15,692	16,737	18, 337	19,487	19,747	20,682	21,742	23,140	23,899	25,079		
FLOODING SOURCE	CROSS SECTION	Big Bear Brook	(continued)	×	ᆸ	Σ	Z	0	щ	Q	ĸ	ß	H	Þ	>	W	×	X	Z	AA	AB		

¹ Feet above confluence with Millstone River

FLOODWAY DATA

BEAR BROOK

TOWNSHIP OF WEST WINDSOR, NJ (Mercer Co.)

FEDERAL EMERGENCY MANAGEMENT AGENCY

BIG

	r	 							 													 _
Z .	INCREASE		0.1	0.1	0.1	0.0	0.2	0.2		0.1	0.0	0.0	0.2	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.2	
BASE FLOOD SURFACE ELEVATION	WITH FLOODWAY NGVD)		82.3	82.6	82.7	84.8	87.3	87.9		67.4	70.0	71.0	74.0	80.3	84.7	87.3	87.3	88.2	91.5	1001	103.4	
BASE F WATER SURFAC	WITHOUT FLOODWAY (FEET		82.2	82.5	82.6	84.8	87.1	87.7		67.3	70.0	71.0	73.8	80.3	84.7	87.3	87.3	88.2	91.5	100.1	103.2	
X	REGULATORY		82.2	82.5	82.6	84.8	87.1	87.7		67.3	70.0	71.0	73.8	80.3	84.7	87.3	87.3	88.2	91.5	100.1	103.2	
	MEAN VELOCITY (FEET PER SECOND)	-	2.6	1.5	1.6	5.3	6.0	1.3		3.3	6.0	8.9	2.3	5.5	1.7	9.0	6.5	3.0	8.9	4.2	1.6	
FLOODWAY	SECTION AREA (SQUARE FEET)		446	797	712	219	1,263	851		160	554	92	230	94	308	592	57	64	28	46	115	
	WIDTH (FEET)		109	87	91	86	290	190		58	110	55	97	104	102	123	21	20	18	30	192	
RCE	DISTANCE ¹		580	1,350	1,657	1,725	3,155	4,412		700	812	1,712	2,422	3,322	3,517	3,697	4,093	4,593	5,533	6,213	6,399	
FLOODING SOURCE	CROSS SECTION	Bear Creek	Ą	М	ບ	Q	ធ	ſΞ	Canoe Brook	Æ	щ	ບ	Q	E	ᄕᅩ	Ŋ	Ħ	н	ט	×	H	

1 Feet above confluence with Big Bear Brook

FLOODWAY DATA

BEAR CREEK AND CANOE BROOK

(Mercer Co.)

TOWNSHIP OF WEST WINDSOR, NJ

FEDERAL EMERGENCY MANAGEMENT AGENCY

ECTION DISTANCE (FEET) SECTION WEAN VELOCITY FIGURATY FLOODWAY FLO	ECTION DISTANCE (FEET) (SQUARE (FEET) RECORD) AT 2,4601 44 110 2.3 62.0 53.83 53.9 55.03 55.03 55.00 1.2 2,4601 115 2.04 11.2 62.0 55.03 55.03 55.0 55.00 1.2 2,4601 115 2.04 11.2 62.0 55.03 55.0 55.0 55.00 1.2 2,4001 24 11.2 62.0 55.03 55.0 55.0 55.00 1.2 2 1.2 62.0 55.03 55.0 55.0 55.0 55.0 55.0 55.0 5	FLOODING SOURCE	JRCE		FLOODWAY		W	BASE FLC WATER SURFACE	FLOOD CE ELEVATION	Z
From Eq. (2.7601) 44 110 2.3 62.0 53.83 53.9 0.2 62.0 54.83 54.9 0.3 62.0 54.83 54.9 0.3 62.0 54.83 54.9 0.3 62.0 54.83 54.9 0.3 62.0 55.93 56.9 0.3 62.0 55.03 56.9 0.3 62.0 56.93 56.9 0.3 62.0 56.93 56.9 0.3 62.0 58.03 58.1 0.3 62.0 59.33 59.4 0.3 62.0 59.4 0.3 62.0 59.33 59.4 0.3 62.0 59.4 0.3 62.0 59.33 59.4 0.3 62.0 59.33 59.4 0.3 62.0 59.63 59.7 0.1 62.0 59.93 60.0 0.3 62.0 59.63 59.7 0.1 62.0 59.93 60.0 0.3 62.0 59.93 60.0 0.3 62.0 59.93 60.0 0.3 62.0 59.93 60.0 0.3 62.0 59.93 59.4 57.0 0.3 62.0 59.93 60.0 0.3 62.0 59.93 59.4 57.0 0.3 62.0 59.94 57.0 0.3 62.3 58.94 57.0 0.3 62.3 58.94 57.0 0.3 62.3 58.94 57.0 0.3 62.3 58.94 60.1 0.3 62.3 58.94 60.1 0.3 62.3 58.94 60.1 0.3 62.3 58.94 60.1 0.3 62.3 58.94 60.1 0.3 62.3 58.94 60.1 0.3 62.3 58.94 60.1 0.3 62.3 58.94 60.1 0.3 62.3 58.94 60.1 0.3 62.3 58.94 60.1 0.3 62.3 58.94 60.1 0.3 62.3 58.94 60.1 0.3 62.3 58.94 60.1 0.3 62.3 58.94 60.1 0.3 62.3 58.94 60.1 0.3 62.3 58.94 60.1 0.3 62.3 58.94 60.1 0.3 62.3 58.94 60.1 0.3 62.3 58.94 60.1 0.3 62.3 62.94 60.1 0.3 62.3 62.94 60.1 0.3 62.94 60.	Aut 110 2.3 62.0 53.83 53.9 0. 2,6761 50 176 1.4 62.0 54.83 54.9 0. 2,6761 50 176 1.4 62.0 55.13 55.9 0. 3,8061 53 169 1.5 62.0 55.13 55.2 0. 5,2091 115 204 1.2 62.0 58.23 58.1 0. 5,2091 115 204 1.7 62.0 58.23 58.1 0. 6,7501 202 375 0.5 62.0 58.3 58.3 0. 7,9401 382 807 0.1 62.0 58.3 58.4 0. 10,8181 163 359 0.3 62.0 59.63 59.4 0. 10,8181 163 359 0.3 62.0 59.63 59.4 0. 11,0818 163 359 0.3 62.0 59.63 59.4 57.0 0. 12,0811 7102 379 1,202	SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY (FEET	WITH FLOODWAY NGVD)	INCREASE
2,4601 44 110 2.3 62.0 53.83 53.9 0.0 2,6761 50 176 1.4 62.0 55.13 55.2 0.0 3,8961 53 169 1.5 62.0 55.13 55.2 0.0 5,2091 115 204 1.2 62.0 56.93 56.9 0.0 5,3001 24 105 1.7 62.0 58.23 58.3 0.0 6,7501 202 375 0.5 62.0 58.23 58.3 0.0 7,9401 382 807 0.1 62.0 58.23 59.4 0.0 10,8181 163 359 0.3 62.0 59.63 59.7 0.0 12,0811 71 163 0.6 62.0 59.93 60.0 0.1 1,1752 136 156 3.8 66.3 56.94 57.0 0.0 2,1702 2,7052 202 623 0.9 66.3 56.94 57.0 0.0 2,7052 202 623 0.9 66.3 56.94 57.0 0.0 2,7052 202 623 0.9 66.3 58.4 58.8 0.0 2,7052 202 623 0.9 66.3 59.34 59.4 0.1 0.0 8,2242 883 1,778 0.3 66.3 65.84 65.8 0.0 8,2242 168 291 1.9 71.2 71.2 71.2 71.2	2,4601 44 110 2.3 62.0 53.83 53.9 0. 2,6761 50 176 1.4 62.0 54.83 54.9 0. 3,8061 53 169 1.5 62.0 55.13 55.2 0. 5,2091 115 204 1.2 62.0 56.93 56.9 0. 5,2091 122 2.1 62.0 56.93 56.9 0. 5,2091 15 204 1.2 62.0 58.03 56.9 0. 5,2091 15 204 1.2 62.0 58.03 58.1 0. 10,8181 163 359 0.1 62.0 59.63 59.4 0. 10,8181 71 163 0.6 62.0 59.63 59.7 0. 11,775 136 156 3.8 66.3 56.94 57.0 0. 1,175 202 623 0.9 66.3 56.94 57.0 0. 2,9442 62 280 2.0 66.3 59.34 59.4 0. 1,178 0.3 66.3 59.94 60.1 0. 5,4092 97 1,708 0.3 66.3 59.94 60.1 0. 5,4092 97 1,708 0.3 66.3 59.94 60.1 0. 6,504 65.8 66.3 59.94 60.1 0.	Bear								
2,4601 44 110 2.3 62.0 53.83 53.9 0. 2,6761 50 176 1.4 62.0 54.83 53.9 0. 3,8061 53 169 1.5 62.0 54.83 54.9 0. 3,8061 53 169 1.5 62.0 56.93 54.9 0. 5,2091 115 204 1.2 62.0 58.73 56.9 0. 6,7501 202 375 0.5 62.0 58.73 58.8 0. 7,9401 90 194 0.9 62.0 59.63 59.4 0. 9,5401 382 807 0.1 62.0 59.63 59.4 0. 10,8181 163 359 0.3 62.0 59.63 59.7 0. 12,0811 71 163 0.6 62.0 59.63 59.7 0. 12,0811 71 163 0.6 62.0 59.93 60.0 0. 1,7052 136 1,202 0.5	2,4601 44 110 2.3 62.0 53.83 53.9 0. 2,6761 50 176 1.4 62.0 54.83 53.9 0. 3,8061 53 169 1.5 62.0 56.93 56.9 0. 5,2091 115 204 1.2 2.1 62.0 58.03 58.3 0. 6,7501 24 105 1.7 62.0 58.73 58.8 0. 7,9401 90 194 0.9 62.0 58.73 58.8 0. 9,5401 382 80.7 0.1 62.0 59.63 59.4 0. 10,8181 163 359 0.3 62.0 59.63 59.7 0. 11,08181 163 359 0.3 62.0 59.63 59.7 0. 12,0811 71 163 0.6 62.0 59.63 59.7 0. 12,0811 71 163 0.6 62.0 59.93 60.0 0. 1,7752 136 1,202 <td>Brook</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td>	Brook								
2,6761 50 176 1.4 62.0 54.83 54.9 0. 3,8061 53 169 1.5 62.0 55.13 55.2 0. 3,8991 60 122 2.1 62.0 56.93 56.9 0. 5,2091 115 204 1.2 62.0 58.03 56.9 0. 6,7501 24 105 1.7 62.0 58.73 58.1 0. 7,9401 90 194 0.9 62.0 59.43 59.4 0. 10,8181 163 359 0.3 62.0 59.63 59.7 0. 12,0811 71 163 0.6 62.0 59.63 59.7 0. 12,0811 71 163 0.6 62.0 59.63 59.7 0. 12,0811 71 163 0.6 62.0 59.63 59.7 0. 12,0811 71 71.20 0.5 66.3 56.94 57.0 0. 2,7052 202 623 0.9	2,6761 50 176 1.4 62.0 54.83 54.9 0. 3,8061 53 169 1.5 62.0 55.13 55.2 0. 3,8061 60 122 2.1 62.0 55.33 55.2 0. 5,2091 115 204 1.2 62.0 58.93 58.9 0. 6,7501 24 105 1.7 62.0 58.73 58.8 0. 7,9401 90 194 0.9 62.0 59.63 59.4 0. 10,8181 163 359 0.3 62.0 59.63 59.7 0. 10,8181 163 359 0.3 62.0 59.63 59.7 0. 11,081 163 0.6 62.0 59.63 59.7 0. 12,081 7102 379 0.5 66.3 56.94 57.0 0. 1,775 2,705 22 22 20 22	A	2,4601	44	110	•	•	m	•	
3,8061 53 169 1.5 62.0 55.13 55.2 0. 3,8991 60 122 2.1 62.0 56.93 56.93 56.9 0. 5,2091 115 204 1.2 62.0 58.03 56.9 0. 6,7501 24 105 1.7 62.0 58.73 58.3 0. 7,9401 90 194 0.9 62.0 58.73 58.8 0. 9,5401 382 807 0.1 62.0 59.63 59.4 0. 10,8181 163 359 0.3 62.0 59.63 59.7 0. 12,0811 71 163 0.6 62.0 59.63 59.7 0. 12,0811 71 163 0.6 62.0 59.93 60.0 0. 12,0811 71 168 3.8 66.3 56.94 57.0 0. 12,0811 71702 0.5 66.3 56.94 57.0 0. 2,9442 62 280 2.0 <td>3,8061 53 169 1.5 62.0 55.13 55.2 0. 3,8991 60 122 2.1 62.0 56.93 56.9 0. 5,2091 115 204 1.2 62.0 56.93 56.9 0. 6,7501 22 105 62.0 58.23 58.1 0. 7,9401 90 194 0.9 62.0 58.73 58.8 0. 10,8181 163 359 0.1 62.0 59.33 59.4 0. 10,8181 163 359 0.3 62.0 59.63 59.7 0. 10,8181 163 359 0.3 62.0 59.93 59.7 0. 12,0811 71 163 0.6 62.0 59.93 60.0 0. 12,0811 71 163 0.6 66.3 56.94 57.0 0. 1,1752 136 156 3.8 66.3 58.4 58.8 0. 2,9442 62 280 2.0 66.3</td> <td>Д</td> <td>2,6761</td> <td>50</td> <td>176</td> <td>•</td> <td>•</td> <td></td> <td>•</td> <td></td>	3,8061 53 169 1.5 62.0 55.13 55.2 0. 3,8991 60 122 2.1 62.0 56.93 56.9 0. 5,2091 115 204 1.2 62.0 56.93 56.9 0. 6,7501 22 105 62.0 58.23 58.1 0. 7,9401 90 194 0.9 62.0 58.73 58.8 0. 10,8181 163 359 0.1 62.0 59.33 59.4 0. 10,8181 163 359 0.3 62.0 59.63 59.7 0. 10,8181 163 359 0.3 62.0 59.93 59.7 0. 12,0811 71 163 0.6 62.0 59.93 60.0 0. 12,0811 71 163 0.6 66.3 56.94 57.0 0. 1,1752 136 156 3.8 66.3 58.4 58.8 0. 2,9442 62 280 2.0 66.3	Д	2,6761	50	176	•	•		•	
3,8991 60 122 2.1 62.0 56.93 56.9 0.0 5,2091 115 204 1.2 62.0 58.03 58.1 0.0 6,7501 24 105 1.7 62.0 58.23 58.3 0.0 7,9401 202 375 0.5 62.0 59.33 59.4 0.0 10,8181 163 359 0.3 62.0 59.63 59.4 0.0 10,8181 163 359 0.3 62.0 59.63 59.7 0.0 10,8181 163 359 0.3 62.0 59.63 59.7 0.0 10,8181 71 163 0.6 62.0 59.63 59.7 0.0 12,0811 71 163 0.6 62.0 59.63 59.7 0.0 12,081 7,102 321 783 0.8 66.3 56.94 57.0 0.0 2,7052 202 623 0.9 66.3 59.34 59.4 0.0 2,4442 62	8,8991 60 122 2.1 62.0 56.93 56.9 0. 5,2091 115 204 1.2 62.0 58.03 56.9 0. 5,3001 24 105 1.7 62.0 58.23 58.1 0. 6,7501 202 375 0.5 62.0 58.73 58.8 0. 7,9401 90 194 0.9 62.0 58.73 58.8 0. 9,5401 382 807 0.1 62.0 59.63 59.4 0. 10,8181 163 359 0.3 62.0 59.63 59.7 0. 12,0811 71 163 0.6 62.0 59.63 59.7 0. 12,0811 71 163 0.6 62.0 59.63 59.7 0. 12,0811 71 163 0.6 62.0 59.93 60.0 0. 2,1702 32 156 3.8 66.3 56.94 57.0 0. 2,9442 62 280 2.0	ບ	3,8061	53	169	•	•	•	•	0.1
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6,7501 202 375 0.5 62.0 58.73 58.8 0. 7,9401 90 194 0.9 62.0 59.33 59.4 0. 9,5401 382 807 0.1 62.0 59.63 59.7 0. 10,8181 163 359 0.3 62.0 59.63 59.7 0. 12,0811 71 163 0.6 62.0 59.63 59.7 0. 12,0811 71 163 0.6 62.0 59.63 59.7 0. 12,0811 71 163 0.6 62.0 59.93 60.0 0. 11,1752 136 1,202 0.5 66.3 56.94 57.0 0. 2,1702 321 783 0.8 66.3 56.94 57.0 0. 2,7052 202 623 0.9 66.3 58.64 58.8 0. 2,9442 62 280 2.0 66.3 59.34 59.34 59.4 0. 4,1242 438 617<	6,7501 202 375 0.5 62.0 58.73 58.8 0. 7,9401 90 194 0.9 62.0 59.33 59.4 0. 9,5401 382 807 0.1 62.0 59.63 59.4 0. 10,8181 163 359 0.3 62.0 59.63 59.7 0. 12,0811 71 163 0.6 62.0 59.63 59.7 0. 12,0811 71 163 0.6 62.0 59.93 60.0 0. 12,0811 71 163 0.6 62.0 59.93 60.0 0. 1,1752 136 1,202 0.5 66.3 56.94 57.0 0. 2,1702 321 783 0.9 66.3 58.34 58.5 0. 2,9442 62 280 2.0 66.3 59.94 60.1 0. 4,1242 438 617 0.9 66.3 59.94 60.1 0. 7,0492 97 159 3.5	ſv.	5,3001	24	105	•	•	•	•	0.1
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Py, 540 ¹ 382 807 0.1 62.0 59.6 ³ 59.7 0. 10,818 ¹ 163 359 0.3 62.0 59.6 ³ 59.7 0. 12,081 ¹ 71 163 0.6 62.0 59.6 ³ 59.7 0. Run 210 ² 379 1,202 0.5 66.3 56.9 ⁴ 57.0 0. 2,170 ² 321 783 0.8 66.3 56.9 ⁴ 57.0 0. 2,705 ² 202 623 0.9 66.3 58.3 ⁴ 58.8 0. 2,944 ² 62 280 2.0 66.3 59.3 ⁴ 59.9 ⁴ 60.1 0. 4,124 ² 438 617 0.9 66.3 59.3 ⁴ 59.9 ⁴ 60.1 0. 5,479 ² 883 1,778 0.3 66.3 65.9 60.5 60.5 60.5 60.5 60.5 60.5 60.5 60.5 60.5 60.5 60.5 60.5 60.5 60.5 60.5 60.5 60.5 60.5 60.5 <	9,540 ¹ 382 807 0.1 62.0 59.6 ³ 59.7 0. 10,818 ¹ 163 359 0.3 62.0 59.6 ³ 59.7 0. 12,081 ¹ 71 163 359 0.3 62.0 59.6 ³ 59.7 0. Run 210 ² 379 1,202 0.5 66.3 56.9 ⁴ 57.0 0. 1,175 ² 136 1,202 0.5 66.3 56.9 ⁴ 57.0 0. 2,170 ² 321 783 0.8 66.3 58.3 ⁴ 58.5 0. 2,705 ² 202 623 0.9 66.3 58.6 ⁴ 58.8 0. 2,442 ² 62 280 2.0 66.3 59.3 ⁴ 59.4 0. 4,124 ² 438 617 0.9 66.3 59.9 ⁴ 60.1 0. 5,479 ² 98 1,778 0.3 66.3 65.8 ⁴ 65.8 0. 7,049 ² 97 159 3.5 66.3 65.8 ⁴ 65.8 0.	 1	7,9401			•	•	9.3	6	0.1
Run 210² 379 0.3 62.0 59.6³ 59.7 0. Run 210² 379 1,20² 0.6 62.0 59.9³ 60.0 0. 1,175² 379 1,20² 0.5 66.3 56.9⁴ 57.0 0. 2,170² 321 783 0.8 66.3 56.9⁴ 57.0 0. 2,705² 202 623 0.9 66.3 58.3⁴ 58.5 0. 2,944² 62 280 2.0 66.3 59.3⁴ 59.4 0. 4,124² 438 617 0.9 66.3 59.3⁴ 59.3⁴ 60.1 0. 5,479² 883 1,778 0.3 66.3 59.9⁴ 60.1 0. 7,049² 97 159 3.5 66.3 65.8⁴ 65.8⁴ 65.8 0. 8,224² 168 291 1.9 71.2 71.2 71.2 0.	Run 210 ² 379 0.3 62.0 59.6 ³ 59.7 0. 12,081 ¹ 71 163 0.6 62.0 59.6 ³ 59.7 0. Run 210 ² 379 1,202 0.5 66.3 56.9 ⁴ 57.0 0. 2,170 ² 321 783 0.8 66.3 56.9 ⁴ 57.0 0. 2,705 ² 202 623 0.9 66.3 58.3 ⁴ 58.5 0. 2,944 ² 62 280 2.0 66.3 58.6 ⁴ 58.8 0. 4,124 ² 438 617 0.9 66.3 59.9 ⁴ 60.1 0. 5,479 ² 883 1,778 0.3 66.3 59.9 ⁴ 60.1 0. 7,049 ² 97 159 3.5 66.3 65.8 ⁴ 65.8 0. 8,224 ² 168 291 1.9 71.2 71.4 0.	u	9,5401	382	807	•	•	9.6	6	0.1
Run 210 ² 379 1,202 0.5 66.3 56.9 ⁴ 57.0 0. 1,175 ² 136 1,202 0.5 66.3 56.9 ⁴ 57.0 0. 2,170 ² 321 783 0.8 66.3 58.3 ⁴ 58.5 0. 2,705 ² 202 623 0.9 66.3 58.3 ⁴ 58.5 0. 2,705 ² 202 623 0.9 66.3 58.3 ⁴ 58.5 0. 2,944 ² 62 280 2.0 66.3 59.3 ⁴ 59.4 0. 4,124 ² 438 617 0.9 66.3 59.9 ⁴ 60.1 0. 5,479 ² 883 1,778 0.3 66.3 66.3 65.8 ⁴ 66.6 0. 8,224 ² 168 291 1.9 71.2 71.2 71.4 0.	Run 210 ² 379 1,202 0.5 66.3 56.94 57.0 0. 1,175 ² 136 1,202 0.5 66.3 56.94 57.0 0. 2,170 ² 321 783 0.8 66.3 56.94 57.0 0. 2,170 ² 321 783 0.8 66.3 58.34 58.5 0. 2,705 ² 202 623 0.9 66.3 58.64 58.8 0. 2,944 ² 62 280 2.0 66.3 59.34 59.4 0. 4,124 ² 438 617 0.9 66.3 59.94 60.1 0. 5,479 ² 97 159 3.5 66.3 65.84 65.8 0. 8,224 ² 168 291 1.9 71.2 71.4 0.	_	10,8181	163	359	•	•	ó		0.1
Run 2102 3.79 1,202 0.5 66.3 56.94 57.0 0. 1,1752 136 156 3.8 66.3 56.94 57.0 0. 2,1702 321 783 0.8 66.3 58.34 57.0 0. 2,7052 202 623 0.9 66.3 58.64 58.5 0. 2,9442 62 280 2.0 66.3 59.34 59.4 0. 4,1242 438 617 0.9 66.3 59.94 60.1 0. 5,4792 883 1,778 0.3 66.3 65.3 65.84 65.8 0. 8,2242 168 291 1.9 71.2 71.2 71.4 0.	Rum 2102 379 1,202 0.5 66.3 56.94 57.0 0. 1,1752 136 156 3.8 66.3 56.94 57.0 0. 2,1702 321 783 0.8 66.3 56.94 57.0 0. 2,1702 321 783 0.9 66.3 58.34 58.5 0. 2,7052 202 623 0.9 66.3 58.64 58.8 0. 4,1242 438 617 0.9 66.3 59.34 60.1 0. 5,4792 883 1,778 0.3 66.3 69.94 60.6 0. 7,0492 97 159 3.5 66.3 65.84 65.8 8,2242 168 291 1.9 71.2 71.2 71.4 0.		12,0811	71	9	•	•	ó	•	•
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	2102 379 1,202 0.5 66.3 56.94 57.0 0 1,1752 136 156 3.8 66.3 56.94 57.0 0 2,1702 321 783 0.8 66.3 58.34 58.5 0 2,7052 202 623 0.9 66.3 58.64 58.8 0 2,9442 62 280 2.0 66.3 59.34 59.4 0 4,1242 438 617 0.9 66.3 59.94 60.1 0 5,4792 883 1,778 0.3 66.3 66.3 65.84 60.6 0 7,0492 97 159 3.5 66.3 65.84 65.8 0 8,2242 168 291 1.9 71.2 71.2 71.4 0									
1,1752 136 156 3.8 66.3 56.94 57.0 0. 2,1702 321 783 0.8 66.3 58.34 58.5 0. 2,7052 202 623 0.9 66.3 58.64 58.8 0. 2,7052 202 623 0.9 66.3 59.34 59.4 0. 4,1242 438 617 0.9 66.3 59.94 60.1 0. 5,4792 883 1,778 0.3 66.3 65.84 65.8 0. 7,0492 97 159 3.5 66.3 65.84 65.8 0. 8,2242 168 291 1.9 71.2 71.4 0.	136 156 3.8 66.3 56.94 57.0 0. 321 783 0.8 66.3 58.34 58.5 0. 202 623 0.9 66.3 58.64 58.8 0. 62 280 2.0 66.3 59.34 59.4 0. 438 617 0.9 66.3 59.94 60.1 0. 883 1,778 0.3 66.3 60.54 60.6 0. 97 159 3.5 66.3 65.84 65.8 0. 168 291 1.9 71.2 71.2 71.4 0.		2102	270	1 202		2	56 94		-
2,170² 321 783 0.8 66.3 58.34 58.5 0. 2,705² 202 623 0.9 66.3 58.64 58.8 0. 2,944² 62 280 2.0 66.3 59.34 59.4 0. 4,124² 438 617 0.9 66.3 59.94 60.1 0. 5,479² 883 1,778 0.3 66.3 60.54 60.6 0. 7,049² 97 159 3.5 66.3 65.84 65.8 65.8 8,224² 168 291 1.9 71.2 71.4 0.	321 783 0.8 66.3 58.34 58.5 0. 202 623 0.9 66.3 58.64 58.8 0. 62 280 2.0 66.3 59.34 59.4 0. 438 617 0.9 66.3 59.94 60.1 0. 883 1,778 0.3 66.3 60.54 60.6 0. 97 159 3.5 66.3 65.84 65.8 0. 168 291 1.9 71.2 71.2 71.4 0.	1	1,1752	136	156		66.3	56.94	57.0	
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	202 66.3 58.64 58.8 0. 62 280 2.0 66.3 59.34 59.4 0. 438 617 0.9 66.3 59.94 60.1 0. 883 1,778 0.3 66.3 60.54 60.6 0. 97 159 3.5 66.3 65.84 65.8 0. 168 291 1.9 71.2 71.2 71.4 0.	r \	2,1702	321	783	•	66.3	•	•	•
2,9442 62 280 2.0 66.3 59.34 59.4 0. 4,1242 438 617 0.9 66.3 59.94 60.1 0. 5,4792 883 1,778 0.3 66.3 60.54 60.6 0. 7,0492 97 159 3.5 66.3 65.84 65.8 0. 8,2242 168 291 1.9 71.2 71.2 71.4 0.	62 280 2.0 66.3 59.34 59.4 0. 438 617 0.9 66.3 59.94 60.1 0. 883 1,778 0.3 66.3 60.54 60.6 0. 97 159 3.5 66.3 65.84 65.8 0. 168 291 1.9 71.2 71.2 71.4 0.	0	2,7052	202	623	•		9	æ	•
$4,124^2$ 438 617 0.9 66.3 66.3 60.94 60.1 0.1 $5,479^2$ 883 $1,778$ 0.3 66.3 60.54 60.6 $0.0.6$ $7,049^2$ 97 159 3.5 66.3 65.84 65.8 65.8 $8,224^2$ 168 291 1.9 71.2 71.2 71.4 0.0	438 617 0.9 66.3 59.94 60.1 0. 883 1,778 0.3 66.3 60.54 60.6 0. 97 159 3.5 66.3 65.84 65.8 0. 168 291 1.9 71.2 71.2 71.4 0.	ы	2,9442	62	280	•		6		•
5,4792 883 1,778 0.3 66.3 60.54 60.6 0. 7,0492 97 159 3.5 66.3 65.84 65.8 0. 8,2242 168 291 1.9 71.2 71.2 71.4 0.	883 1,778 0.3 66.3 60.54 60.6 0. 97 159 3.5 66.3 65.84 65.8 0. 168 291 1.9 71.2 71.2 71.4 0.	Fc.	4,1242	438	617	•		6	60.1	•
7,049 ² 97 159 3.5 66.3 65.8 ⁴ 65.8 0. 8,224 ² 168 291 1.9 71.2 71.2 71.4 0.	97 159 3.5 66.3 65.8 ⁴ 65.8 0. 168 291 1.9 71.2 71.4 0.	rh.	5,4792	883	•	•		•	•	•
8,224 ² 168 291 1.9 71.2 71.2 71.4 0.	168 291 1.9 71.2 71.2 71.4 0.		7,0492	76	159	•		•	•	•
			224	168	291	•	.	•	•	•

FLOODWAY DATA

of backwater effects from Stony Brook

 3 Elevation computed without consideration of backwater effects from the Millstone River

(MERCER CO.)

FEDERAL EMERGENCY MANAGEMENT AGENCY

 $^2{
m Feet}$ above corporate limits

TOWNSHIP OF WEST WINDSOR, NJ

LITTLE BEAR BROOK AND DUCK POND RUN

FLOODING SOURCE	RCE		FLOODWAY			BASE F WATER SURFAC	BASE FLOOD SURFACE ELEVATION	7.
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY (FEET	WITH FLOODWAY NGVD)	INCREASE
Duck Pond Run								
(continued)								
ט	8,9441	124	260	2.2	73.2	73.2	73.2	0.0
×	9,8141	185	315	1.8	75.0	75.0	75.2	0.2
ц	9,9111	118	254	2.2	75.5	75.5	75.6	0.1
×	10,2761	66	266	2.1	76.6	76.6	76.7	0.1
Z	10,5491	127	626	9.0	79.0	79.0	79.2	0.2
0	10,8791	210	891	0.4	79.0	79.0	79.2	0.2
ц	11,0241	260	1,145	0.3	9.62	79.6	79.8	0.2
а	11,6841	319	1, 139	0.3	79.6	9.62	79.8	0.2
ρĽ	12,5391	114	530	0.7	79.7	79.7	79.9	0.2
ഗ	13,5991	132	337	1.1	80.0	80.0	80.2	0.2
E	15, 1491	155	265	1.3	81.3	81.3	81.4	0.1
n	15,5421	48	9/	4.7	82.2	82.2	82.2	0.0
^	15,7981	59	213	1.2	85.1	85.1	85.1	0.0
W	16,5381	83	249	1.0	85.2	85.2	85.2	0.0
×	17,4531	129	108	2.3	87.1	87.1	87.2	0.1
¥	18,8331	75	189	1.3	9.68	9•68	89.8	0.2
2	19,0641	135	597	0.4	92.0	92.0	92.0	0.0
Stony Brook						•		
A	7702	750/453	4,737	2.2	57.8	57.8	57.9	0.1
ф	2,3342	695/203	4,863	2.1	58.0	58.0	58.1	0.1
υ	3,8842	735/203	5,673	1.8	58.1	58.1	58.2	0.1

 $^{1} \label{teet} Feet above corporate limits \\ ^{2} \mbox{Feet above confluence with Millstone River}$

 3 width/width within corporate limits

FEDERAL EMERGENCY MANAGEMENT AGENCY

TOWNSHIP OF WEST WINDSOR, NJ (MERCER CO.)

FLOODWAY DATA

DUCK POND RUN AND STONY BROOK

7.	INCREASE		0.1	0.1	0.0	0.0	0.1	0.1	0.2	0.2	0.1	0.1	0.2	0.2	0.2	0.2	0.2	0.2		
BASE FLOOD SURFACE ELEVATION	WITH FLOODWAY NGVD)		58.3	58.5	59.1	61.1	62.4	62.8	63.6	64.3	66.3	9.99	 70.5	71.4	71.9	72.5	73.1	73.9		
BASE F WATER SURFAC	WITHOUT FLOODWAY (FEET		58.2	58.4	59.1	61.1	62.3	62.7	63.4	64.1	66.2	66.5	 70.3	71.2	71.7	72.3	72.9	73.7	5, 7,7	
Ĭ.	REGULATORY		58.2	58.4	59.1	61.1	62.3	62.7	63.4	64.1	66.2	66.5	70.3	71.2	71.7	72.3	72.9	73.7		
	MEAN VELOCITY (FEET PER SECOND)		1.9	2.3	4.0	2.9	1.4	1.7	1.5	1.4	1.5	1.2	9.0	0.7	0.5	0.8	0.8	9.0		
FLOODWAY	SECTION AREA (SQUARE FEET)		5,344	4,449	2,541	3,539	7,474	5,741	6,468	7,042	6,811		1, 132	850	1,146	•	658	919		
	WIDTH (FEET)		780/203	830/103	550/20 ³	540/303	1,130/203	1,030/203	1,065/303	1,470/303	1,295/403	1,765/353	415	424	730	470/2953	290/2403	375/1503		
CE	DISTANCE		5,7621	7,4721	7,8351	8,7401	11,2701	12,8701	14,7901	16,8401	20,4701	22,1201	14.8102	15.8102	16,940 ²	18,0502	19,8852	21,390 ²		
FLOODING SOURCE	CROSS SECTION	Stony Brook	(continued)	ы	ĹŦ4	ტ	ш	н	ט	×		Σ	Assumpink creek	t pr	a c) _) <u>F</u>) E4		

1 Feet above confluence with Millstone River

2Feet above corporate limits
3width/width within corporate limits

TOWNSHIP OF WEST WINDSOR, NJ FEDERAL EMERGENCY MANAGEMENT AGENCY

(Mercer Co.)

FLOODWAY DATA

STONY BROOK AND ASSUNPINK CREEK

	+	·											
Z	INCREASE		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
BASE FLOOD SURFACE ELEVATION	WITH FLOODWAY NGVD)		70.8	71.7	74.7	75.9	76.5	76.6	6.97	77.6	78.1	80.2	
BASE F WATER SURFAC	WITHOUT FLOODWAY (FEET		70.8	711.7	74.7	75.9	76.5	9.92	6.97	77.6	78.1	80.2	
M.	REGULATORY		70.8	71.7	74.7	75.9	76.5	9.92	6.92	77.6	78.1	80.2	
	MEAN VELOCITY (FEET PEK SECOND)		0.3	0.3	0.7	9.0	0.4	0.3	0.2	0.4	9.0	0.3	
FLOODWAY	SECTION AREA (SQUARE FEET)		1,140	1,176	481	290	777	1,195	1,145	428	313	757	
	WIDTH (FEET)		650	530	374	410	363	417	300	228	220	350	
RCE	1 DISTANCE		1,860	2,580	3,580	4,465	4,575	5,040	6,105	6,875	7,410	7,510	
FLOODING SOURCE	CROSS SECTION	Bridegroom Run	A	щ	υ	Ω	ខា	Ēu	ŋ	н	н	p	

1 Feet above confluence with Assunpink Creek

FLOODWAY DATA

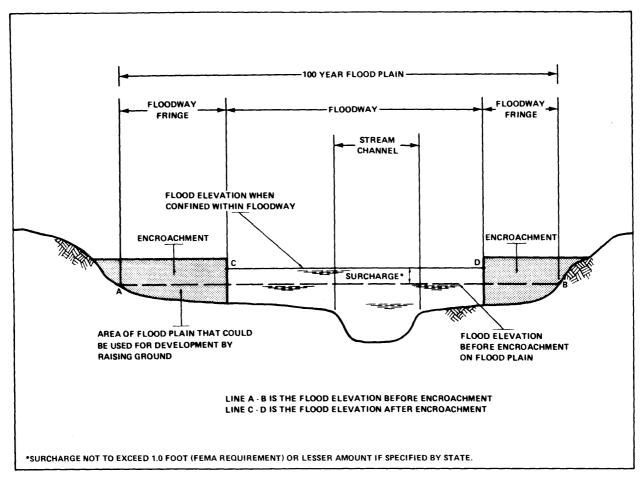
BRIDGROOM RUN

TABLE 2

TOWNSHIP OF WEST WINDSOR, NJ

(Mercer Co.)

FEDERAL EMERGENCY MANAGEMENT AGENCY



FLOODWAY SCHEMATIC

Figure 4

criteria. This process includes the determination of reaches, Flood Hazard Factors (FHFs), and flood insurance zone designations for each flooding source affecting the Township of West Windsor.

5.1 Reach Determinations

Reaches are defined as lengths of watercourses having relatively the same flood hazard, based on the average weighted difference in water-surface elevations between the 10- and 100-year floods. This difference does not have a variation greater than that indicated in the following table for more than 20 percent of the reach.

Average Difference Between	
10- and 100-Year Floods	<u>Variation</u>
Less than 2 feet	0.5 foot
2 to 7 feet	1.0 foot
7.1 to 12 feet	2.0 feet
More than 12 feet	3.0 feet

The locations of the reaches determined for the flooding sources of the Township of West Windsor are shown on the Flood Profiles (Exhibit 1) and are summarized in the Flood Insurance Zone Data Table (Table 3).

5.2 Flood Hazard Factors

The FHF is the FEMA device used to correlate flood information with insurance rate tables. Correlations between property damage from floods and their FHFs are used to set actuarial insurance premium rate tables based on FHFs from 005 to 200.

The FHF for a reach is the average weighted difference between the 10-and 100-year flood water-surface elevations expressed to the nearest 0.5 foot, and shown as a three-digit code. For example, if the difference between water-surface elevations of the 10- and 100-year floods is 0.7 foot, the FHF is 005; if the difference is 1.4 feet, the FHF is 015; if the difference is 5.0 feet, the FHF is 050. When the difference between the 10- and 100-year water-surface elevations is greater than 10.0 feet, accuracy for the FHF is to the nearest foot.

5.3 Flood Insurance Zones

After the determination of reaches and their respective FHFs, the entire incorporated area of the Township of West Windsor was divided into zones, each having a specific flood potential or hazard. Each zone was assigned one of the following flood insurance zone designations:

Zone A: Special Flood Hazard Areas inundated by the 100-year flood, determined by approximate methods; no base flood elevations shown or FHFs determined.

Zones A1, A2, A3, Special Flood Hazard Areas inundated by the 100-year A4, A5 and A7: flood, determined by detailed methods; base flood elevations shown, and zones subdivided according to FHF.

Zone B: Areas between the Special Flood Hazard Area and the limits of the 500-year flood, including areas of the 500-year flood plain that are protected from the 100-year flood by dike, levee, or other water control structure; also, areas subject to certain types of 100-year shallow flooding where depths are less than 1.0 foot; and areas subject to 100-year flooding from sources with drainage areas less than 1 square mile. Zone B is not subdivided.

Zone C: Areas of minimal flooding.

		BETWEEN '	ELEVATION DIFFERENCE ² BETWEEN 1.0% (100-YEAR) FLOOD AND	CE ² OOD AND	i		BASE FLOOD3
FLOODING SOURCE	PANEL	10% (10 YR.)	2% (50 YR.)	0.2% (500 YR.)	± ±	ZONE	(NGVD)
Millstone River							
Reach 1	02	-1.9	9.0-	+2.6	020	A4	Varies
Reach 2	02,03,04,						
	90,20	-2.4	8.0-	+2.3	025	A5	Varies
Biq Bear Brook							
Reach 1	04	-3.3	-1.3	+3.2	035	A7	Varies
Reach 2	04	6.0-	-0-3	+0.8	010	A2	Varies
Reach 3	04,05,08	-1.7	-0.7	+1.5	015	A3	Varies
Reach 4	80	-1.8	-0.5	+0.8	020	A4	Varies
Reach 05	80	-2.6	-0.4	+0.7	025	A5	Varies
Bear Creek							
Reach 1	80	-2.5	-0.3	+0.4	025	A5	Varies
Reach 2	80	-1.0	9.0-	9.0+	010	A2	Varies
Canoe Brook							
Reach 1	04	-0.7	-0.2	+0.5	005	A1	Varies

¹Flood Insurance Rate Map Panel

FEDERAL EMERGENCY MANAGEMENT AGENCY

TOWNSHIP OF WEST WINDSOR, NJ

(Mercer Co.)

FLOOD INSURANCE ZONE DATA

MILLSTONE RIVER, BIG BEAR BROOK, BEAR CREEK, CANOE BROOK

²Weighted Average

Rounded to the nearest foot - see map

		ELE BETWEEN	ELEVATION DIFFERENCE ² BETWEEN 1.0% (100-YEAR) FLOOD AND	CE ² OOD AND			BASE FLOOD
FLOODING SOURCE	PANEL	10% (10 YR.)	2% (50 YR.)	0.2% (500 YR.)	FHF	ZONE	(NGVD)
Duck Pond Run Reach 1	20'90	6.0-	-0.2	9*0+	010	A2	Varies
Stony Brook Reach 1	01,03,06	-2.2	-0.7	+2.7	020	A4	Varies
Assunpink Creek Reach 1	05,09,10	<u>.</u>	-0.3	9•0+	010	A2	Varies
Bridegroom Run Reach 1	08,09,10	-0.5	-0.1	+0•3	900	A1	Varies
		,					

 $^{^{\}rm l}{\rm Flood}$ Insurance Rate Map Panel $^{\rm 2}{\rm Weighted}$ Average $^{\rm 3}{\rm Rounded}$ to the nearest foot – see map

FEDERAL EMERGENCY MANAGEMENT AGENCY

TOWASHIP OF WEST WINDSOR, NJ

(Mercer Co.)

FLOOD INSURANCE ZONE DATA

ASSUNPINK CREEK, BRIDEGROOM RUN STONY BROOK, DUCK POND RUN,

Table 3, "Flood Insurance Zone Data," summarizes the flood elevation differences, FHFs, flood insurance zones, and base flood elevations for the flooding sources studied in detail in the Township of West Windsor.

5.4 Flood Insurance Rate Map Description

The Flood Insurance Rate Map for the Township of West Windsor is, for insurance purposes, the principal result of the Flood Insurance Study. This map (published separately) contains the official delineation of flood insurance zones and base flood elevation lines. Base flood elevation lines show the locations of the expected whole-foot water-surface elevations of the base (100-year) flood. This map is developed in accordance with the latest flood insurance map preparation guidelines published by the FEMA.

6.0 OTHER STUDIES

Flood Insurance Studies have been published for the Townships of Washington, Lawrence and Hamilton (References 5, 6 and 14). Flood Insurance Studies are currently being prepared for the Town of Plainsboro and the Townships of Princeton and East Windsor (References 13, 17 and 18). The study for the Township of West Windsor will be in exact agreement with the contiguous studies mentioned above.

In 1973, a study of the Millstone River was made as part of a study of the flood hazard areas in the Raritan River basin (Reference 19). The results of that study will be compatible with the results of this study. In 1967, a study was also prepared for Stony Brook as a part of the study of the flood hazard areas in the Raritan River basin (Reference 20). The results of that study were compared with the 50- and 100-year flood levels developed in this study. Differences in flood levels were attributed to differences in methods of analysis.

The NJDEP, in cooperation with the USGS, made a study of the magnitude and frequency of floods in New Jersey and the effects of urbanization (Reference 3). The study has been used as a basis for determining flood flows on ungaged streams in New Jersey. The study has also been used to determine approximate 100-year flood elevations in conjunction with graphical depth-discharge-frequency relations for coastal and non-coastal plain sites in New Jersey.

A study by the USGS of the extent and frequency of floods in the upper Millstone River basin provided data for the Millstone River, Big Bear Brook and Bear Creek (Reference 21).

The effects of tropical storms on flood flows in the Millstone River, Stony Brook, Duck Pond Run, Big Bear Brook, Bear Creek, Little Bear Brook, Upper

Bear Swamp and Assumpink Creek were studied in USGS Sepcial Report 37 (Reference 22). Some of the data in Special Report 37 indicate greater flood flows than previously recorded.

A study of the extent and frequency of inundation of the flood plains of the Millstone River, Stony Brook and Lake Carnegie has been prepared by the USGS (Reference 23).

A Flood Hazard Analysis has been prepared for the Assumpink Creek watershed SCS (Reference 7).

This study is authoritative for purposes of the Flood Insurance Program, and the data presented here either supersede or are compatible with previous determinations.

7.0 LOCATION OF DATA

Information concerning the pertinent data used in preparation of this study can be obtained by contacting the Natural and Technological Hazards Division, Federal Emergency Management Agency, Regional Director, Region II Office, 26 Federal Plaza, Room 19-100, New York, New York 10278.

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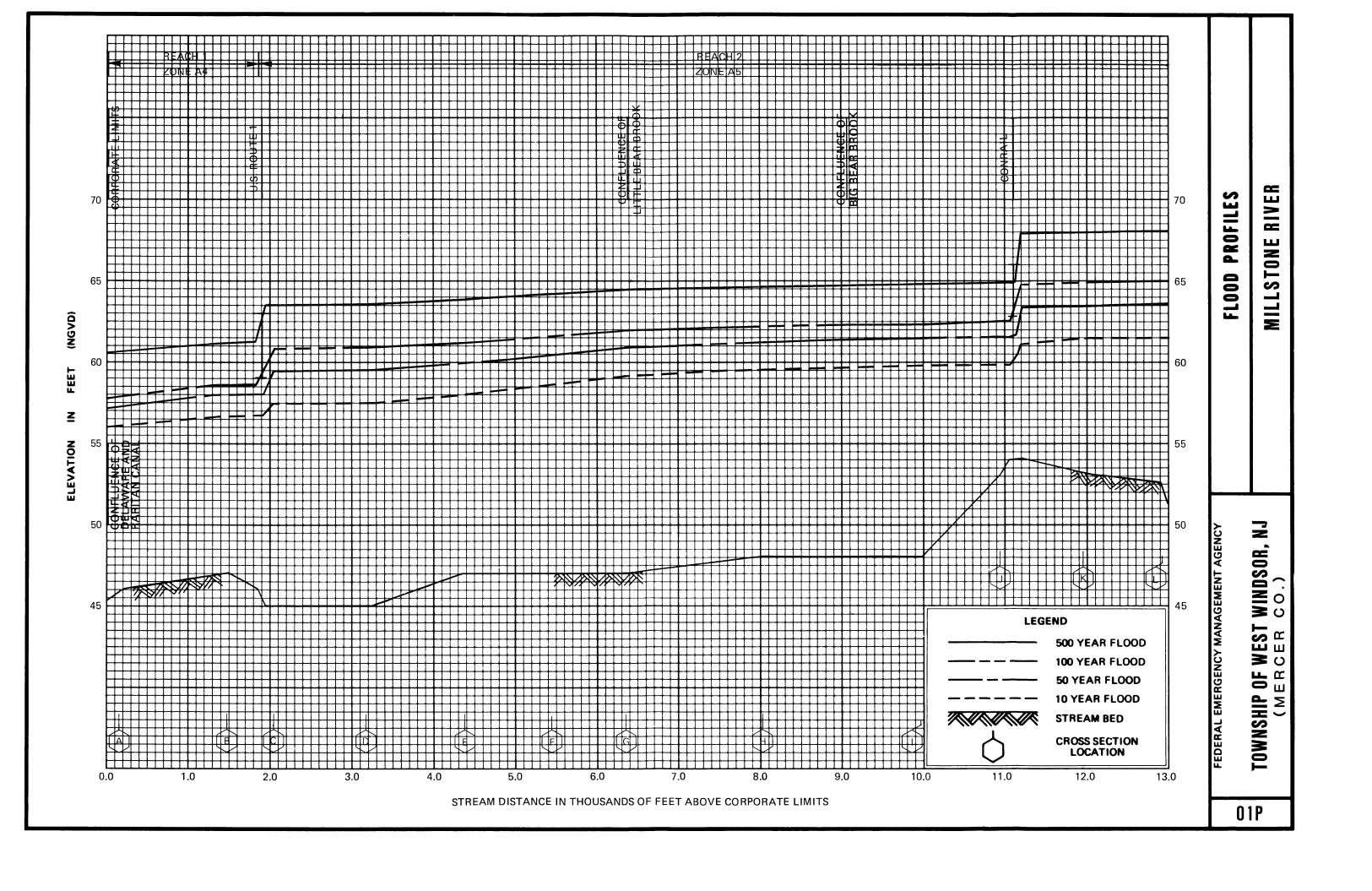
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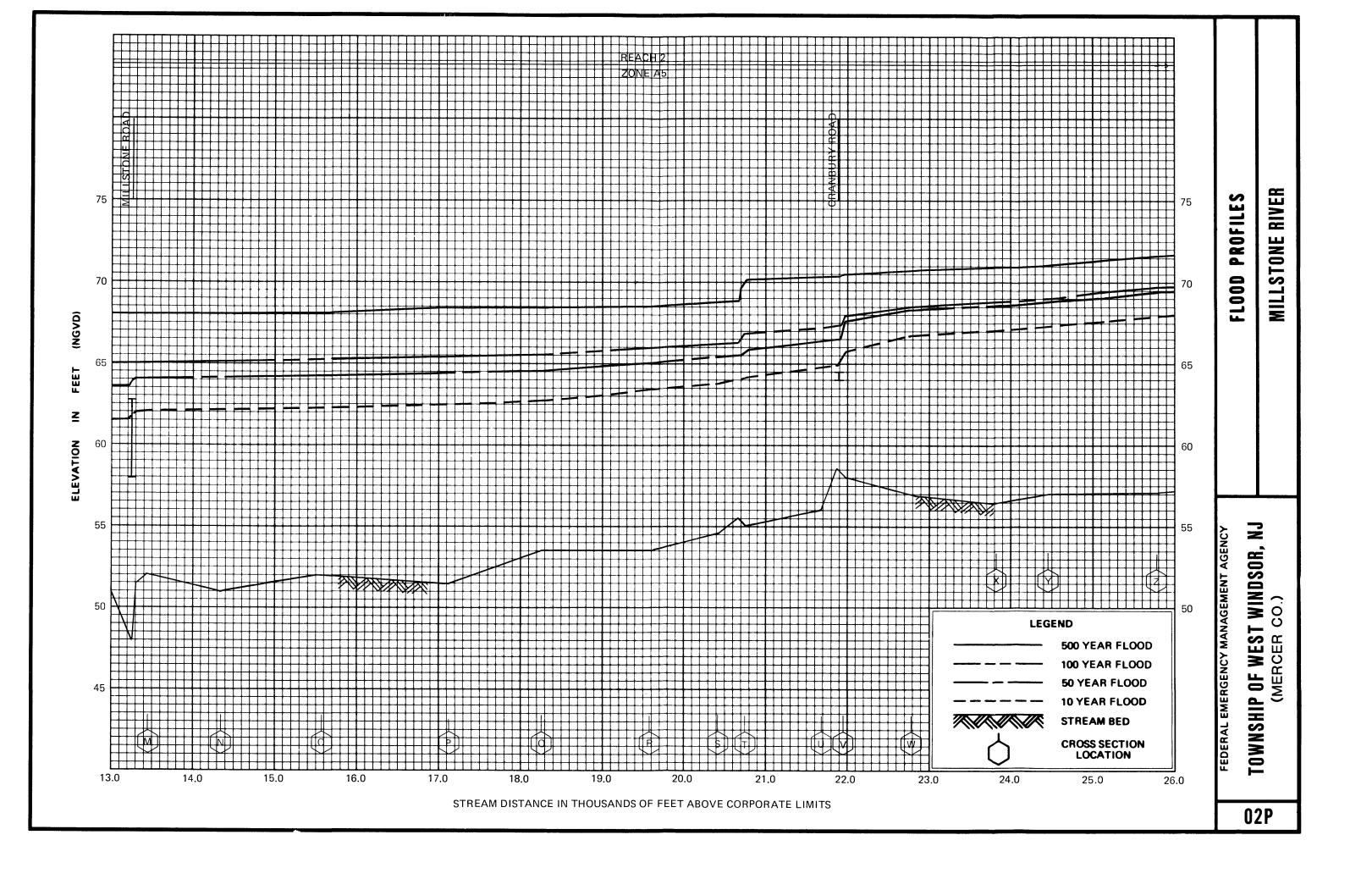
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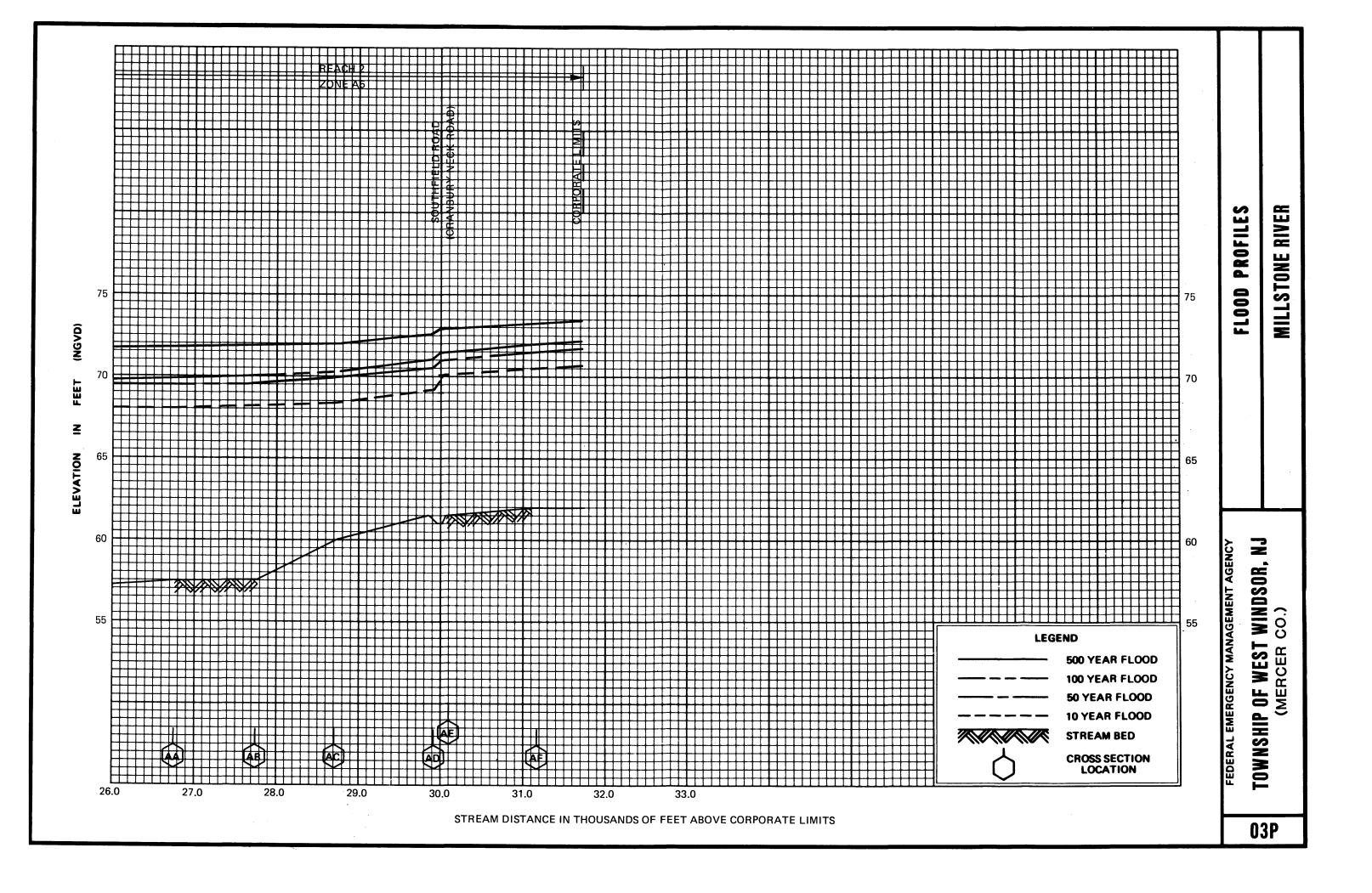
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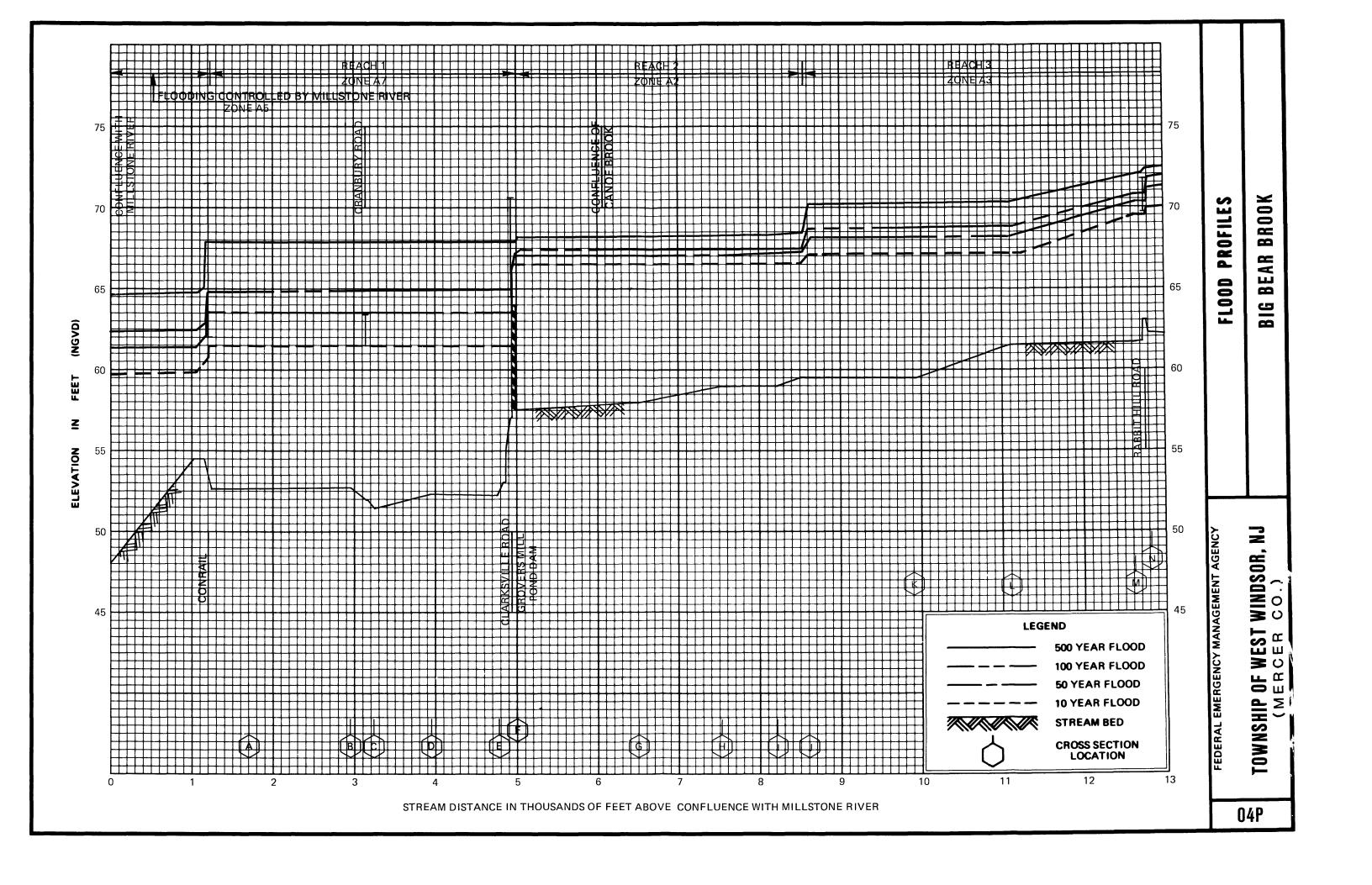
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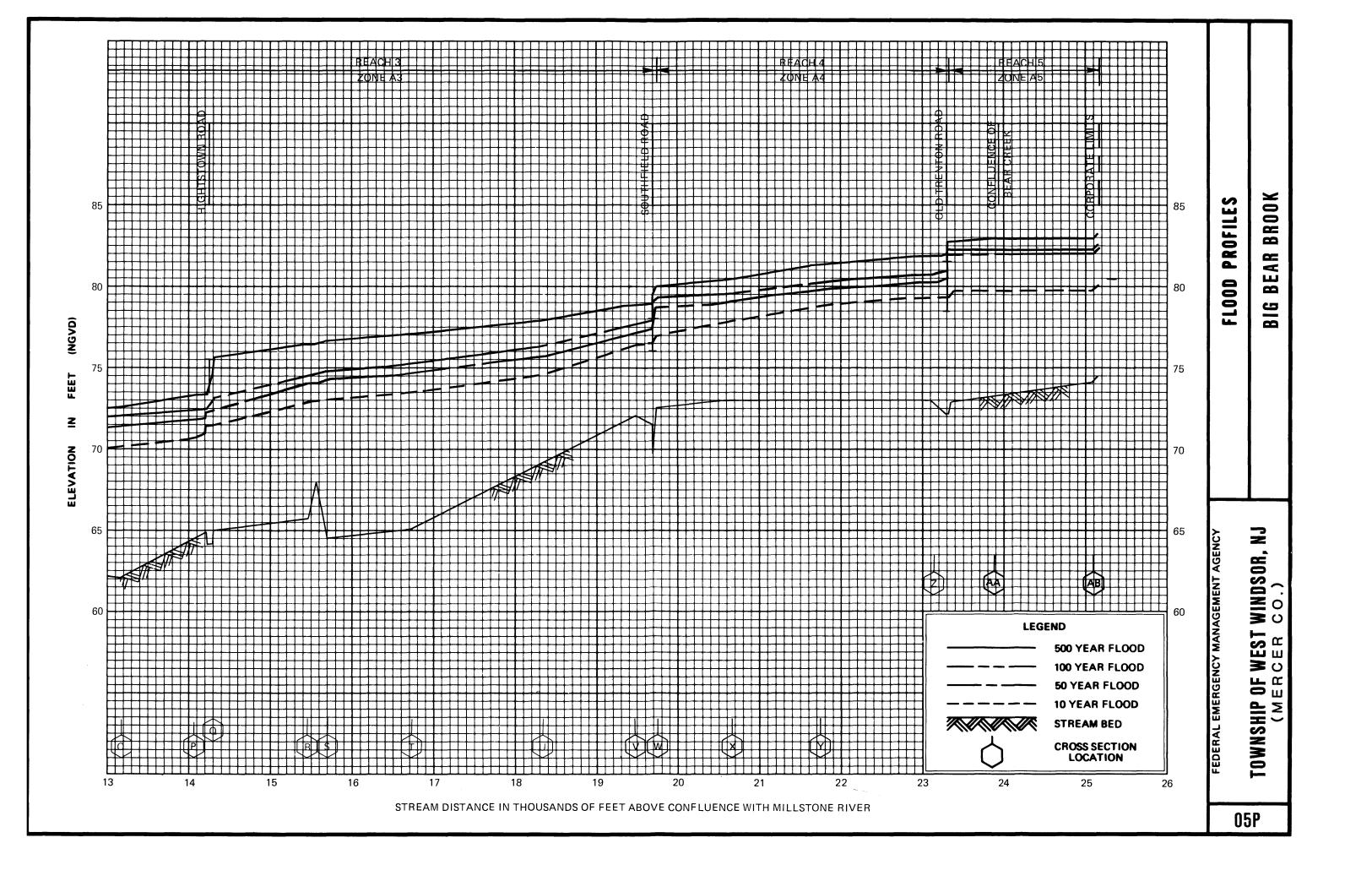
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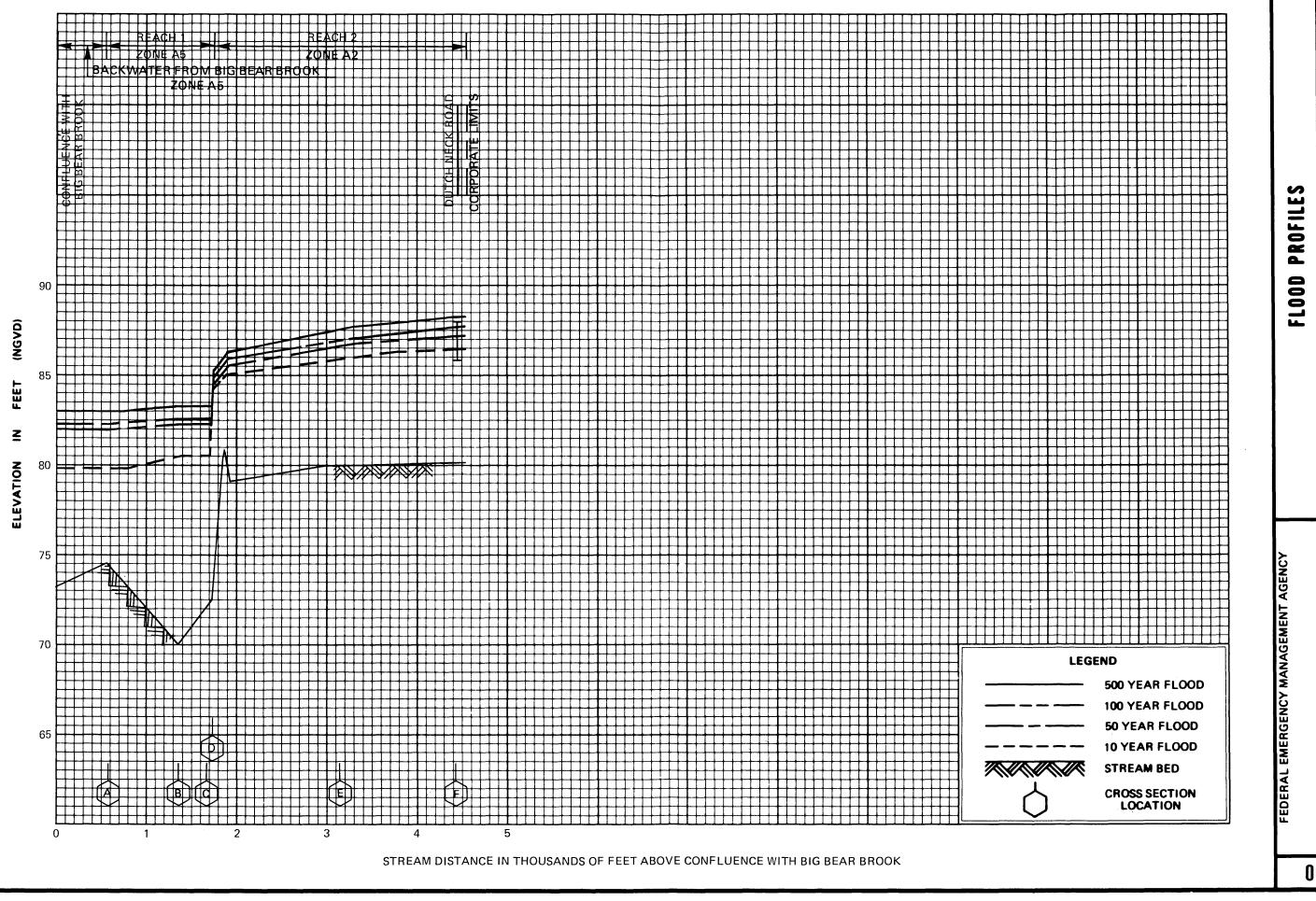








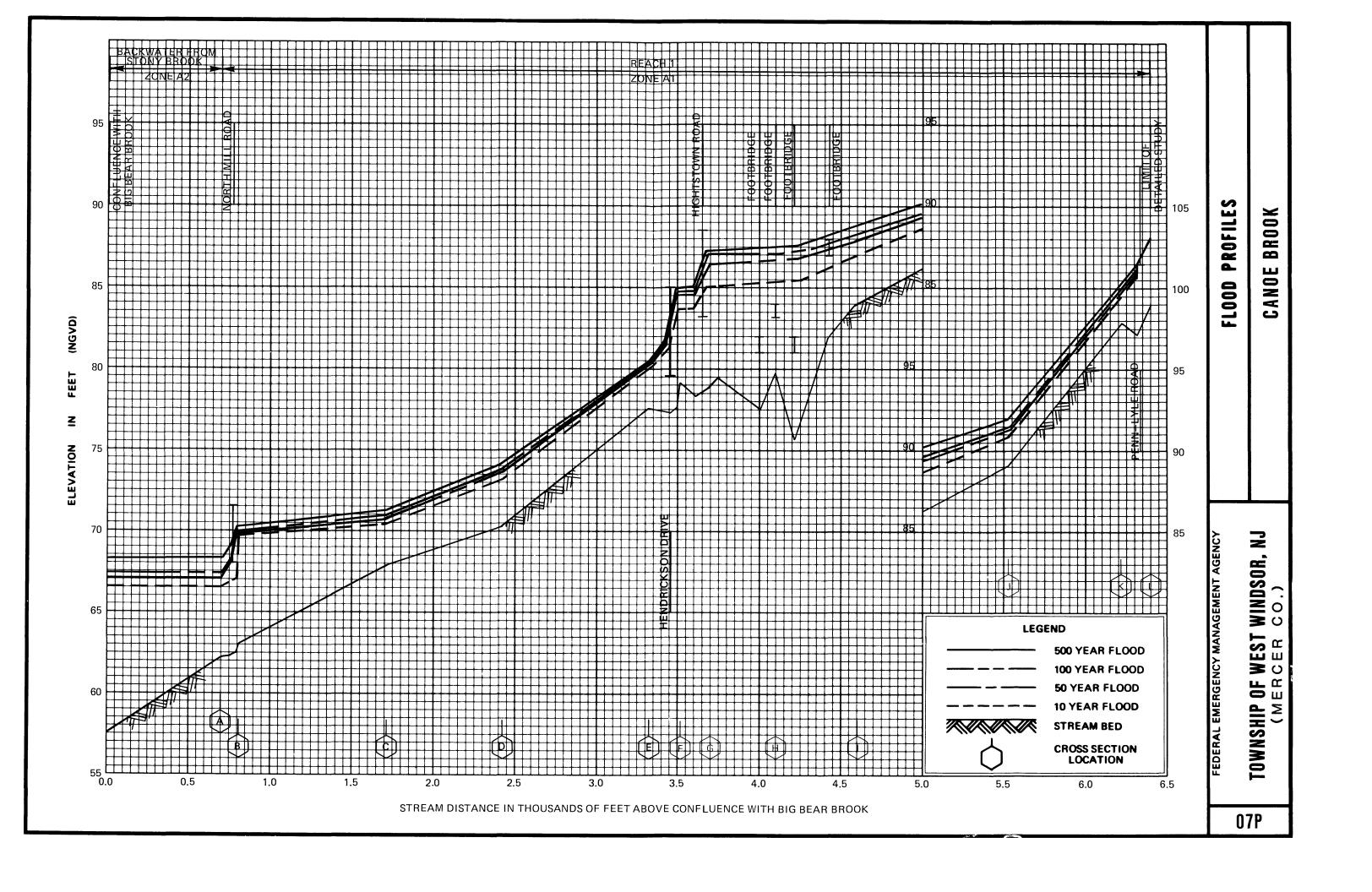


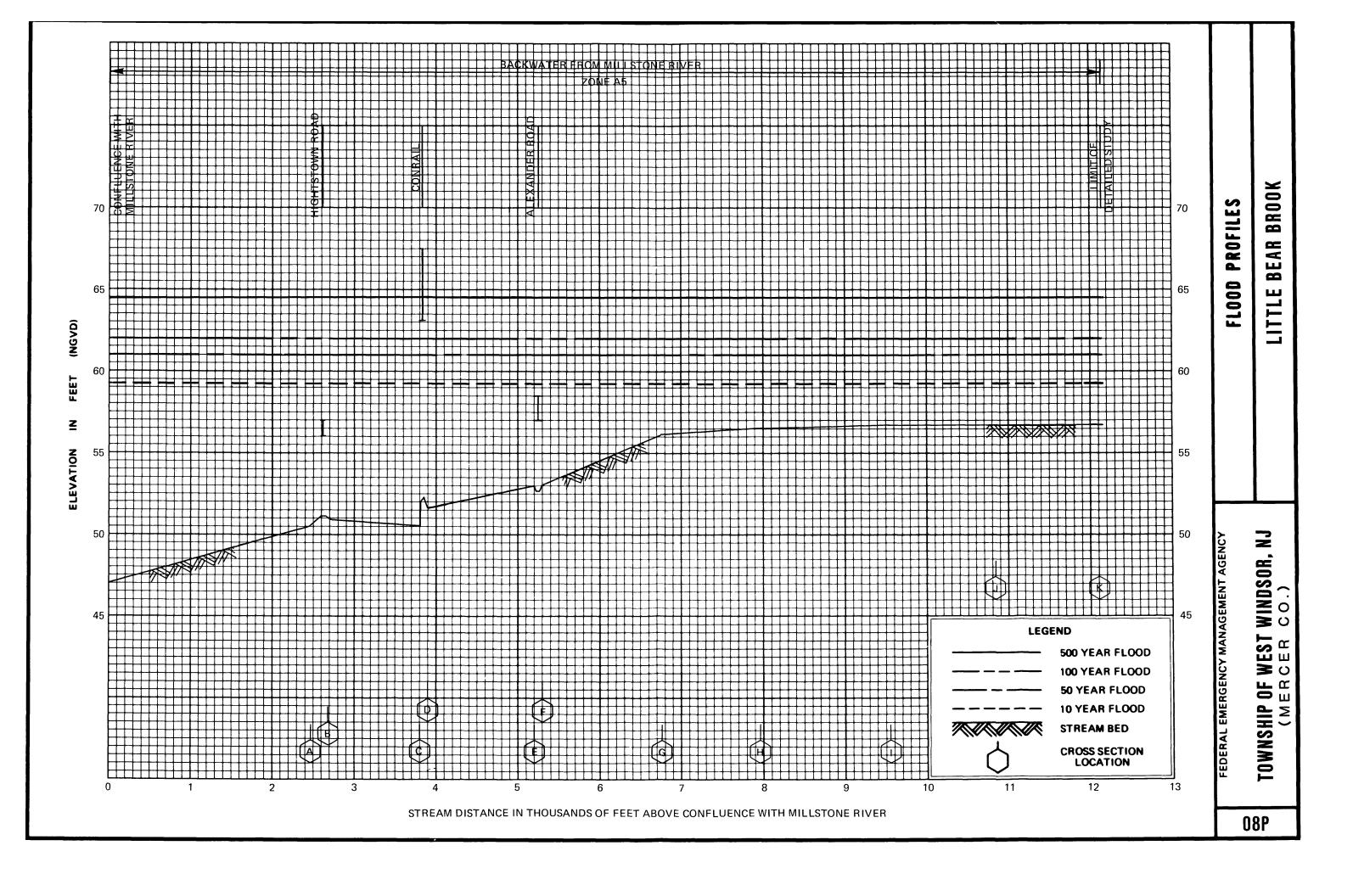


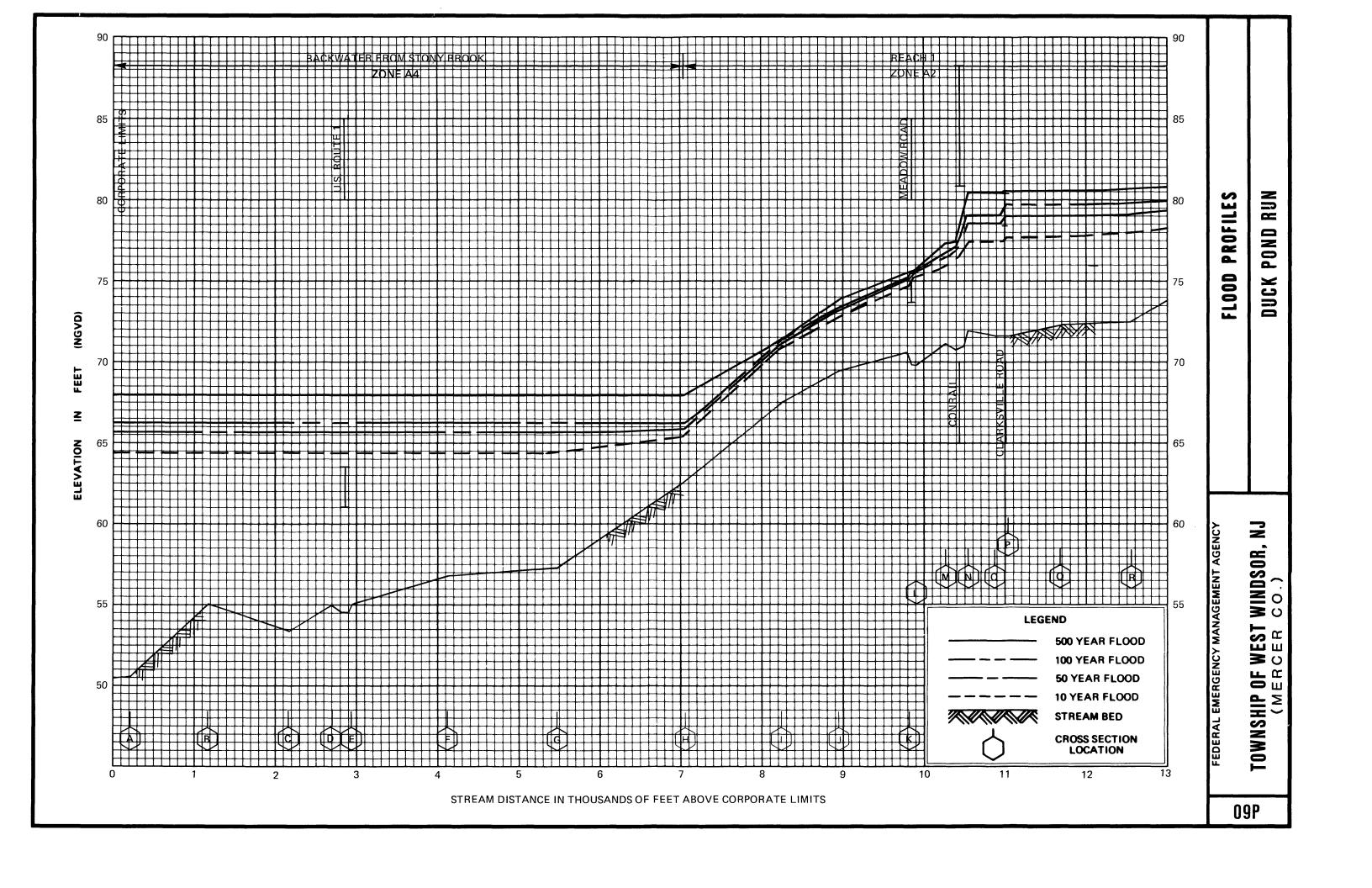
BEAR CREEK

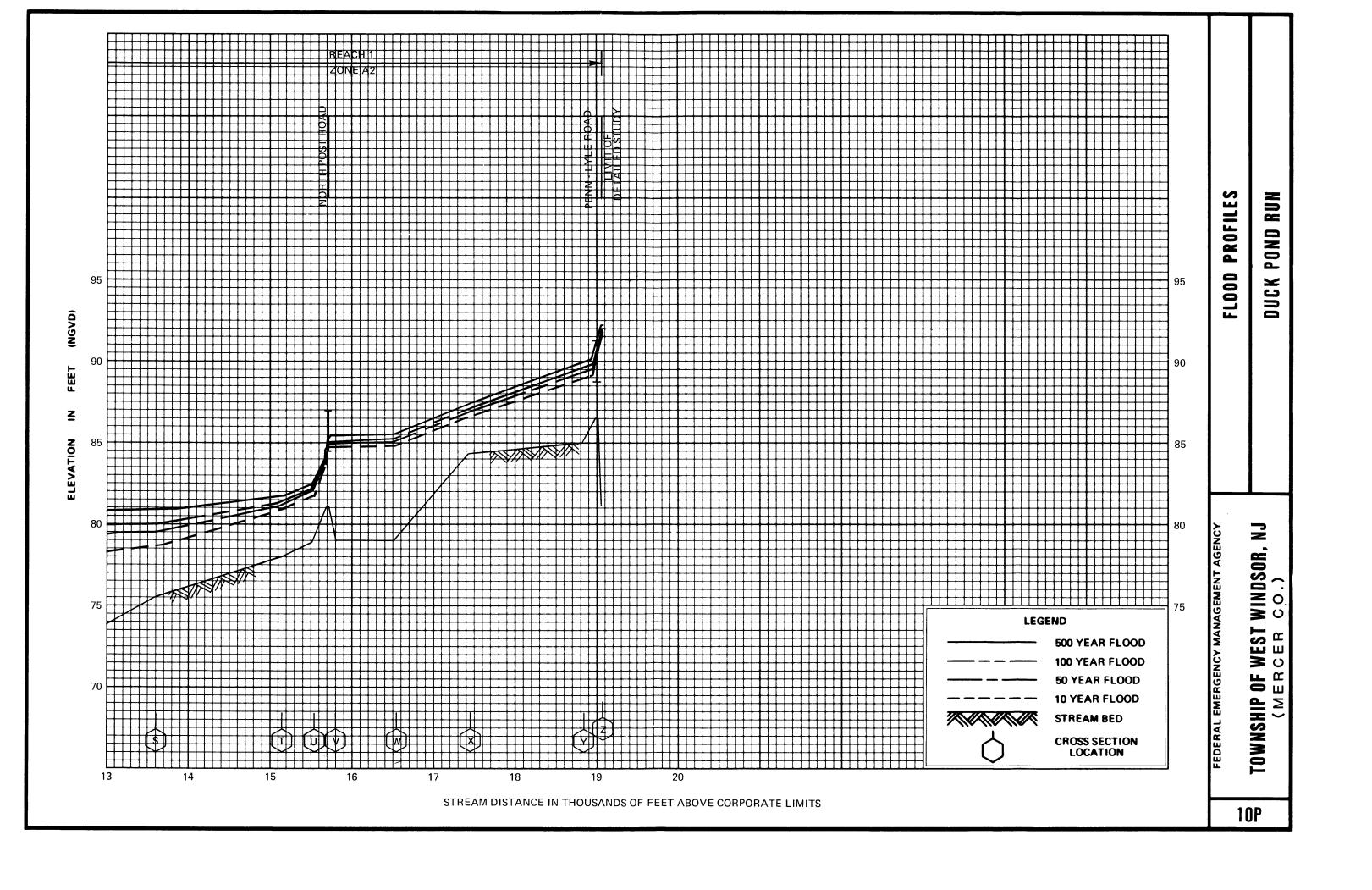
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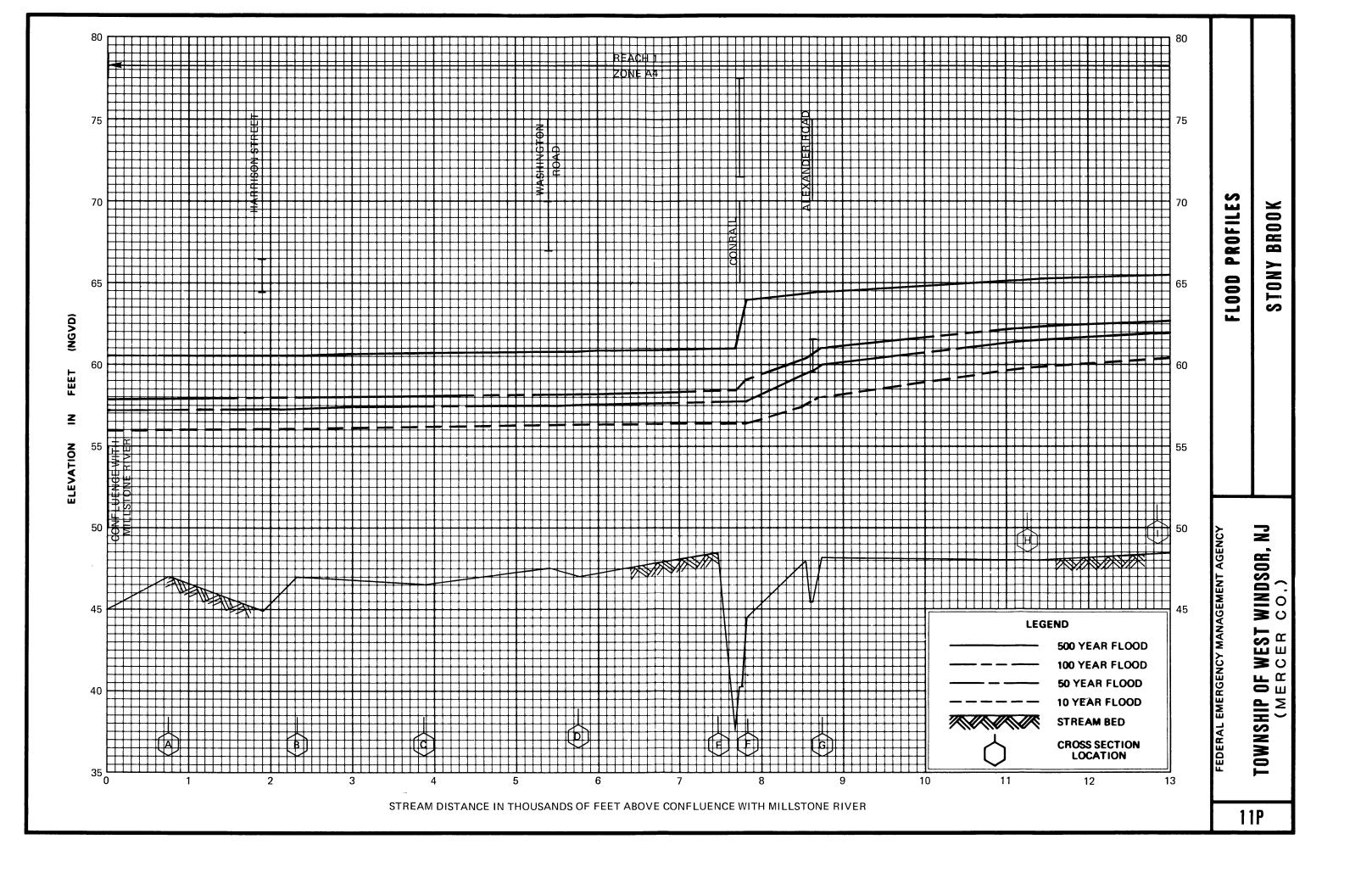
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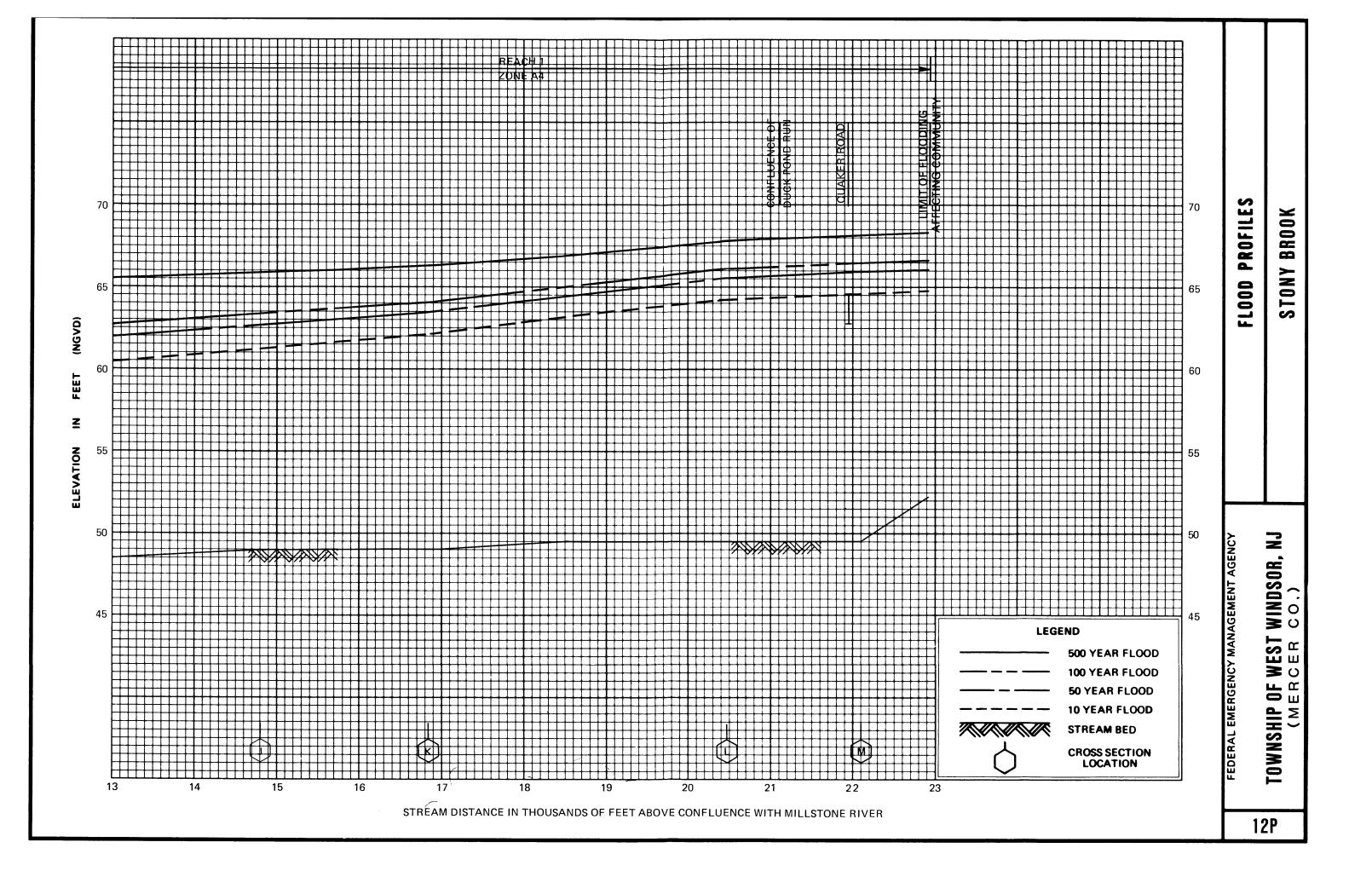


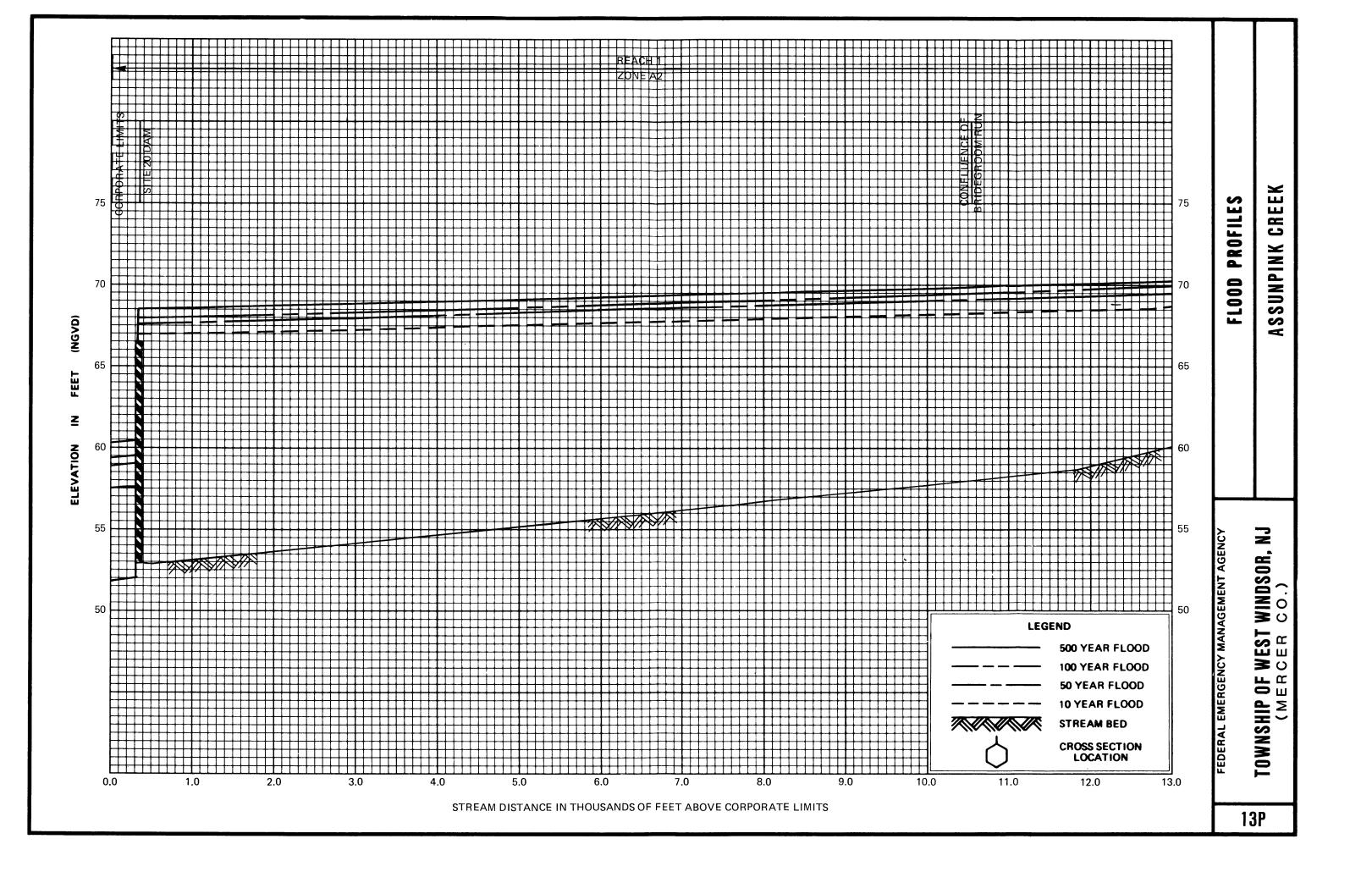


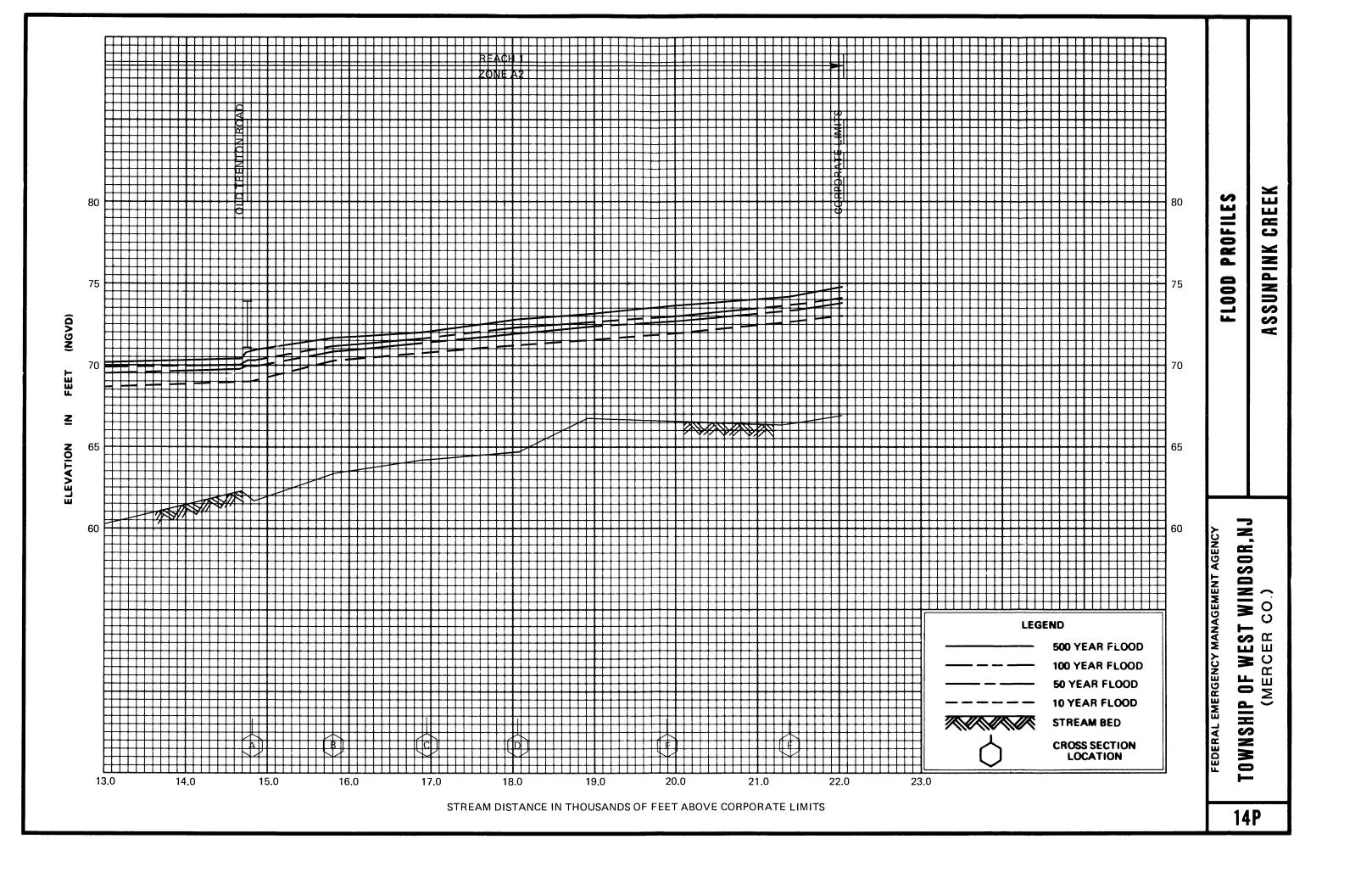


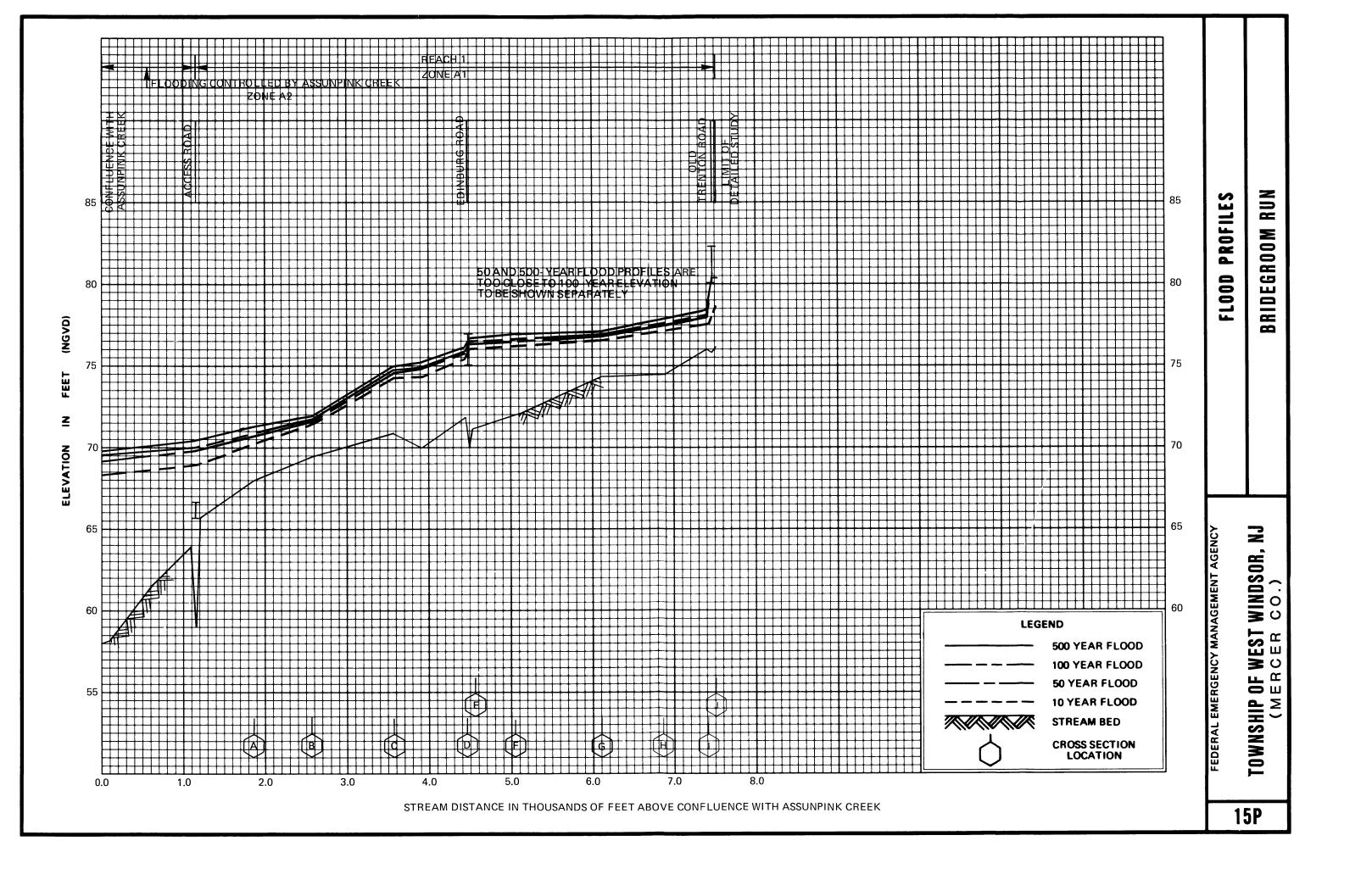












West Windsor Build Out

FEMA WSE Conversion

22 October 2014

INPUT

State Plane, NAD83 2900 - New Jersey, U.S. Feet Vertical - NGVD29 (Vertcon94), U.S. Feet

OUTPUT

State Plane, NAD83 2900 - New Jersey, U.S. Feet Vertical - NAVD88, U.S. Feet

West Windsor 100YR

1/2

Northing/Y: 541719.235 Easting/X: 456227.35 Elevation/Z: 62

Convergence: -0 04 59.91021 Scale Factor: 0.999901473

Combined Factor: 0.999903703

Easting/X: 456227.350
Elevation/Z: 60.921

Northing/Y: 541719.235

Convergence: -0 04 59.91021 Scale Factor: 0.999901473

Combined Factor: 0.999903754

Grid Shift (U.S. ft.): X/Easting = 0.0, Y/Northing = 0.0

West Windsor 500YR

2/2

Northing/Y: 541719.235 Easting/X: 456227.35 Elevation/Z: 64.5

Convergence: -0 04 59.91021 Scale Factor: 0.999901473 Combined Factor: 0.999903583 Northing/Y: 541719.235 Easting/X: 456227.350 Elevation/Z: 63.421

Convergence: -0 04 59.91021 Scale Factor: 0.999901473 Combined Factor: 0.999903635

Grid Shift (U.S. ft.): X/Easting = 0.0, Y/Northing = 0.0